



I CERTIFY THAT THIS WAS PREPARED BY ME
OR UNDER MY DIRECT SUPERVISION

PERMIT SET

MCH RENOVATION PROJECT, GMHA 007-2014
GMHA FAMILY BIRTH CENTER
850 GOVERNOR CAMACHO ROAD, OKA, TAMUNING, GUAM 96913
GUAM MEMORIAL HOSPITAL AUTHORITY
GENERAL NOTES AND STRUCTURAL DESIGN CRITERIA

PROJECT TITLE: OWNER: SHEET TITLE:

MARK DATE DESCRIPTION
DATE : 2024.11.15
PROJECT NO : 144052
DRAWN BY : ASC
CHECKED BY : ASC
COPYRIGHT :

DWG NO:

S0.1

GENERAL STRUCTURAL NOTES

- SLAB ELEVATIONS SHALL BE COORDINATED WITH ARCHITECTURAL DRAWINGS.
- BUILDING DIMENSIONS SHALL BE AS PER ARCHITECTURAL DRAWINGS UNLESS SHOWN OTHERWISE
- TYPICAL DETAILS SHOWN ON SDC SHEETS SHALL APPLY TO ALL DRAWINGS UNLESS NOTED OTHERWISE.
- UNLESS NOTED OTHERWISE, REFER TO DRAWINGS OTHER THAN STRUCTURAL FOR FINISHES, SLOPES, DEPRESSIONS, OPENING CURBS, STAIRS, RAMPS, TRENCHES, EQUIPMENT AND LOCATIONS AND EXTENT OF SUCH CONDITIONS.
- CONTRACTOR TO COORDINATE ALL NEW WORK WITH EXISTING SITE CONDITIONS AND REPORT ANY DISCREPANCIES TO THE ENGINEER PRIOR TO CONSTRUCTION.
- DETAILS OR CONDITIONS NOT FULLY DEVELOPED ON STRUCTURAL DOCUMENTS ARE SIMILAR TO DEVELOPED DETAILS.
- SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR WATERPROOFING, DAMP-PROOFING, AND DRAINAGE REQUIREMENTS.
- REFER TO GEOTECHNICAL REPORT FOR SITE CONDITIONS EXCAVATION, SHORING REQUIREMENTS, UNDERPINNING, BACKFILL BEHIND WALLS AND SUBDRAINAGE PREPARATIONS.
- ALL BUILDING FOUNDATION PLANS AND ROOF PLANS TO BE COORDINATED WITH GENERAL NOTES AND TYPICAL DETAILS AS APPLICABLE.
- STRUCTURES HAVE BEEN DESIGNED TO BE STABLE AND SELF SUPPORTING AFTER THE CONSTRUCTION IS COMPLETE, IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY FOR THE BUILDING'S STABILITY DURING CONSTRUCTION. THIS RESPONSIBILITY ALSO INCLUDES BUT IS NOT LIMITED TO METHOD AND SEQUENCE OF ERECTION, TEMPORARY SHORING AND TEMPORARY BRACING.
- IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO FOLLOW ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION.
- THE REFERENCE DATUM FOR ALL ELEVATIONS IN THIS STRUCTURAL PLAN SET IS BASED FROM THE DATUM USED IN THE CIVIL PLAN SET.

SPECIAL INSPECTION

- SPECIAL INSPECTION IN ACCORDANCE WITH 2009 IBC CHAPTER 17 IS REQUIRED ON THE FOLLOWING PORTIONS OF THE WORK:
CONCRETE REINFORCING STEEL
SOILS STRUCTURAL STEEL
REFER TO THE STATEMENT OF SPECIAL INSPECTIONS FOR MORE SPECIFIC REQUIREMENTS.

CONCRETE

- CAST IN PLACE CONCRETE:
FOUNDATION $f_c = 4,000$ psi
SLAB ON GRADE $f_c = 4,000$ psi
WALLS, COLUMNS, BEAMS, SLABS $f_c = 4,000$ psi
CONCRETE TOPPING OVER METAL DECK $f_c = 4,000$ psi
OTHERS $f_c = 3,000$ psi
** UNLESS NOTED OTHERWISE ON PLANS **
- CONCRETE REINFORCING CLEAR COVER SHALL BE AS FOLLOWS:
CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3 INCHES
CONCRETE EXPOSED TO EARTH OR WEATHER:
NO. 6 OR LARGER BARS 2 INCHES
NO. 6 OR SMALLER BARS 1.5 INCHES
CONCRETE NOT EXPOSED TO EARTH OR IN CONTACT WITH GROUND:
SLAB OR WALLS 0.75 INCHES
BEAMS OR COLUMN PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS 1.5 INCHES
- ALL CONCRETE DIMENSIONS SHOWN ARE MINIMUM DIMENSIONS. CONTRACTOR TO REVIEW FORMING, REINFORCING, REINFORCING DETAILS AND ANY EMBEDDED ITEMS AND DETERMINE PRIOR TO FABRICATION OF ANY REINFORCING, PLACEMENT REQUIREMENTS AND CLEARANCES.
- CONCRETE MASONRY UNIT (CMU):
COMPOSITE CMU (MIN.) $f_m = 1,500$ psi

CONCRETE AND MASONRY REINFORCEMENT

- ALL CONCRETE REINFORCING SHALL CONFORM TO THE REQUIREMENTS OF ASTM A706. ASTM A615 STEEL MAY BE USED IF IT CONFORMS TO ALL REQUIREMENTS OF ACI 318-08 SECTION 21.1.5.
- REINFORCING SHALL EXTEND CONTINUOUS FOR THE DIMENSION SHOWN.
- NO WELDING OF ANY REINFORCING IS PERMITTED, UNLESS SPECIFICALLY STATED ON THE PLANS. REINFORCEMENT TO BE WELDED TO MEET THE REQUIREMENTS OF ASTM A706.
- LOCATE ALL REINFORCING AS SHOWN ON DRAWINGS AND FASTEN SECURELY.
- ALL REINFORCING TO TERMINATE WITH STANDARD HOOKS AS SHOWN ON PLANS. ALL STIRRUPS AND TIES TO BE CLOSED WITH 135 DEGREE BENDS.

POST INSTALLED ANCHORS

- SEE SPECIFICATIONS SECTION 03 15 00.

FOUNDATION

- UNLESS DIFFERENT VALUES ARE PROVIDED IN THE PROJECT GEOTECHNICAL REPORT, THE FOLLOWING SOIL PROPERTIES SHALL BE ASSUMED:
DL + LL 4,000 psf
DL + LL + WIND OR EARTHQUAKE 6,000 psf
COEFFICIENT OF FRICTION 0.35
PASSIVE SOIL RESISTANCE 250 psf/ft
(SEE EARTHWORK SPECIFICATIONS FOR EARTHWORK, FILL MATERIALS, COMPACTION REQUIREMENTS AND BASECOURSE.)
- THE CONTRACTOR SHALL RETAIN A SOIL ENGINEER TO PREPARE A GEOTECHNICAL REPORT TO VERIFY ASSUMED VALUES HEREIN PRIOR TO COMMENCING ANY WORK.
- ALL GRADING SHALL BE DONE IN ACCORDANCE WITH THE CONTOURS AND DIMENSIONS INDICATED. SUBGRADE SHALL BE SLIGHTLY SLOPED TO PROVIDE PROPER SURFACE DRAINAGE AND TO AVOID SURFACE PONDING.

- EXCAVATION FOR NEW FOUNDATION ELEMENTS AND CONCRETE SLABS ON GROUND SHALL EXTEND DOWN TO A MINIMUM OF 24 INCHES BELOW THE BOTTOM OF THE FOUNDATION ELEMENTS SUCH THAT THE UNDERLYING WHITISH LIMESTONE IS EXPOSED, WITH ALL REDDISH BROWN SILTY SOIL BEING REMOVED. FOUNDATION ELEMENTS SHALL NOT BEAR ON BOULDERS. THE EXPOSED TOP SURFACE OF THE LIMESTONE SHALL BE SCARIFIED TO APPROXIMATELY 12 INCHES DEEP. MOISTURE CONDITIONED AND RECOMPACTED TO AT LEAST 95 PERCENT OF THE MAXIMUM DRY DENSITY. THE EXCAVATION CAN THEN BE BACKFILLED WITH APPROPRIATE MATERIAL AS DESCRIBED BELOW IN LAYERS NOT TO EXCEED 12 INCHES AND MOISTURE CONDITIONED AND THOROUGHLY COMPACTED TO A MINIMUM OF 95 PERCENT OF THE MAXIMUM DRY DENSITY.
- FOOTING EXCAVATION AND RECOMPACTION, WHERE REQUIRED, SHALL BE INSPECTED BY THE GEOTECHNICAL ENGINEER TO VERIFY CONDITION OF SOIL BEARING CAPACITY PRIOR TO PLACEMENT OF FOUNDATION FORMS AND REBAR. WHERE UNSATISFACTORY SOILS ARE ENCOUNTERED, THEY SHALL BE OVEREXCAVATED AND REPLACED WITH A LEAN CONCRETE OR CEMENT GROUT OR AS RECOMMENDED BY THE GEOTECHNICAL ENGINEER.
- FILL AND BACKFILL MATERIAL SHALL BE FREE OF ROCK FRAGMENTS OR LUMPS WITH MAXIMUM DIMENSION LESS THAN 3 INCHES, NON-EXPANSIVE WITH A PLASTICITY INDEX LESS THAN OR EQUAL TO 6 PERCENT AND A LIQUID LIMIT OF NOT MORE THAN 25 PERCENT, AND WITH NO MORE THAN 25 PERCENT PASSING A NO. 200 MESH SIEVE. UNSUITABLE MATERIALS SUCH AS DEBRIS AND ORGANICS SHALL NOT BE USED. ON SITE EXCAVATED SILTY SANDY LIMESTONE GRAVEL SOIL MEETING THE ABOVE REQUIREMENTS MAY BE USED AS BACKFILL MATERIAL.
- CONCRETE SLABS ON GROUND SHALL BEAR ON EITHER NATURAL LIMESTONE OR BACKFILL MATERIAL PLACED AND PREPARED AS DESCRIBED ABOVE, AND SHALL BE UNDERLAIN BY A MINIMUM OF 6 INCHES OF AGGREGATE BASE COURSE, PLACED AND COMPACTED TO AT LEAST 95 PERCENT OF ITS MAXIMUM DRY DENSITY, WITH A UNIFORM AND NON-YIELDING SURFACE.

STEEL

- DETAIL, FABRICATE, AND ERECT STRUCTURAL STEEL IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (LATEST EDITION AND SUPPLEMENTS).
- J ANCHOR BOLTS: ASTM F1554 GRADE 36.
- ALL STEEL BARS & PLATES SHALL BE ASTM A36 UNLESS OTHERWISE NOTED.
- ALL WIDE FLANGE SHAPES SHALL BE ASTM A992 (GRADE 50).
- ALL HSS SECTIONS SHALL BE ASTM A500; GRADE B.
- ALL PIPES TO BE ASTM A53; GRADE B.
- ALL THREADED RODS: ASTM A36 OR ASTM A572; GRADE 50.
- BOLTED CONNECTIONS, UNLESS NOTED OTHERWISE: 3/4-INCH DIAMETER A325-X BOLTS.
- INSTALL HIGH STRENGTH BOLTS IN ACCORDANCE WITH SECTION 8 OF THE SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM HIGH STRENGTH BOLTS, 2009 EDITION.
- PROVIDE REVELED WASHERS ON ALL CONNECTION TO SLOPING FLANGES OF W SECTIONS AND CHANNELS WHERE SLOPE EXCEEDS 1:20.
- ANCHOR RODS SHALL BE THREADED ANCHOR RODS WITH NUT. THE EMBEDDED NUT SHALL BE TACK WELDED TO THE ANCHOR ROD TO PREVENT ROTATION DURING TIGHTENING.
- BOLT HOLES IN STEEL SHALL BE "STANDARD" (1/16-INCH LARGER IN DIAMETER THAN THE NOMINAL BOLT SIZE, UNLESS OTHERWISE NOTED).
- WELDING ELECTRODES (FILLER METAL) E70XX (70KSI), WITH EXACT FILLER METAL SELECTED BY THE FABRICATOR.
- WELD LENGTHS CALLED FOR ON THE PLANS ARE THE NET EFFECTIVE LENGTH REQUIRED, WHERE LENGTH OF WELD IS NOT SHOWN IT SHALL BE FULL LENGTH OF THE JOINT.
- COMPLETE PENETRATION WELDS SHALL BE MADE WITH PROPER BACKING WHEREVER POSSIBLE. AFTER WELDING, REMOVE BACKING BARS AND GRIND SMOOTH. FULL PENETRATION WELDS MADE WITHOUT PROPER BACKING SHALL HAVE THE ROOT GOUGED BEFORE WELDING IS STARTED FROM THE OTHER SIDE EXCEPT AS PROVIDED IN AWS D1.1.
- ALL BUTT AND GROOVE WELDS SHALL BE FULL PENETRATION, UNLESS NOTED OTHERWISE.
- ALL SPLICING OF MEMBERS SHALL BE AS SHOWN ON THE DRAWINGS. ANY SPLICING OF STEEL MEMBERS PROPOSED BY THE STEEL FABRICATOR SHALL BE SHOWN ON SHOP DRAWINGS AND APPROVED BY THE ENGINEER PRIOR TO FABRICATION.
- ALL ANCHOR BOLTS SHALL BE EMBEDDED AS SHOWN ON THE DRAWINGS.
- MINIMUM PLATE THICKNESS IN 3/8 UNLESS OTHERWISE NOTED. MINIMUM WELD ID 1/4 INCH UNLESS OTHERWISE NOTED.
- ALL STEEL FABRICATION AND DETAILS TO COMPLY WITH MOST STRINGENT OF THE LATEST EDITION OF AISC CODE, AWS CODE, AND THE 2009 IBC.
- ALL WELDING TO BE BY AWS CERTIFIED WELDERS AND SHALL CONFORM TO ALL 2009 IBC AND AWS REQUIREMENTS. ALL WELDERS SHALL BE PRE-QUALIFIED BY THE PROJECT WELDING INSPECTOR FOR THE WELD TYPES AND POSITION USED IN THE PROCEDURES THEY WILL BE PERFORMING.
- UNLESS NOTED OTHERWISE, ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED.

STRUCTURAL DESIGN CRITERIA

- REFERENCES
IBC 2009 AISC 360-05
ASCE 7-05 ACI 530-08
ACI 318-08 PCI DESIGN HANDBOOK 6TH EDITION
- LOADS
A. DEAD LOAD
SELF WEIGHT OF THE STRUCTURE
CONCRETE 150 pcf
STEEL 490 pcf

- B. LIVE LOAD
DESIGN LOADS:
PARTITION/HVAC/PIPING FOR FLOORS & ROOF 25 psf
ROOF LIVE LOAD 20 psf
FLOOR/DECK/BALCONY 80 psf
- C. WIND
ASCE 7-05
DESIGN WIND SPEED = 170 mph
EXPOSURE = C
IMPORTANCE FACTOR = 1.15
- D. SEISMIC
SEISMIC DESIGN ASCE 7-05
 $S_s = 1.5$ TRANSITION PERIOD = 12 SEC.
 $S_1 = 0.6$ SITE CLASS C
IMPORTANCE FACTOR = 1.5
SEISMIC DESIGN CATEGORY D
- SEISMIC FORCE RESISTING SYSTEMS:
MAIN BUILDING - R/C SMRF
 $R = 8$ $OMEGA = 3$ $C_d = 5.5$
ELEVATOR/STAIR CORE - SPECIAL R/C BEARING WALLS
 $R = 5$ $OMEGA = 2.5$ $C_d = 5.5$

IBC 2009 / ASCE 7-05 LOAD COMBINATIONS

LRFD (LOAD AND RESISTANCE FACTOR DESIGN)

pg 308 IBC 2009	D	H	F	L	Lr/R	W	E
16-1	1.4		1.4				
16-2b	1.2	1.6	1.2	1.6	0.5		
16-3a	1.2			0.5	1.6		
16-3b	1.2				1.6	0.8	
16-4	1.2			0.5	0.5	1.6	
16-5	1.2			0.5			1
	(1.2 + 0.2S _{as})			0.5			ρ
	(1.2 + 0.2S _{as})			0.5			Ω_e
16-6	0.9	1.6				1.6	
16-7	0.9	1.6					1
	(0.9 - 0.2S _{as})	1.6					ρ
	(0.9 - 0.2S _{as})	1.6					Ω_e
HOR CANT SEISMIC							-0.2 VERT

ASD (ALLOWABLE STRESS DESIGN)

pg 308 IBC 2009	D	H	F	L	Lr/R	W	E
16-8	1		1				
16-9	1	1	1	1			
16-10	1	1	1		1		
16-11	1	1	1	0.75	0.75		
16-12a	1	1	1			1	
16-12b	1	1	1				0.7
	(1 + 0.14S _{as})	1	1				0.7p
ASIF = 1.2	(1 + 0.14S _{as})	1	1				0.7 Ω_e
16-13a	1	1	1	0.75	0.75	0.75	
16-13b	1	1	1	0.75	0.75		0.525
	(1 + 0.10S _{as})	1	1	0.75	0.75		0.525p
ASIF = 1.2	(1 + 0.10S _{as})	1	1	0.75	0.75		0.525 Ω_e
16-14	0.6	1				1	
16-15	0.6	1					0.7
	(1 + 0.14S _{as})	1					0.7p
ASIF = 1.2	(1 + 0.14S _{as})	1					0.7 Ω_e
HOR CANT SEISMIC							-0.2 VERT

D - DEAD LOAD
L - LIVE LOAD
Lr - ROOF LIVE LOAD

W - WIND LOAD
E - SEISMIC LOAD
H - LATERAL EARTH PRESSURE

F - WELL DEFINED FLUID
T - SELF STRAINING FORCES FROM TEMPERATURE
CHANGE, CREEP, SHRINKAGE

IF SHEET IS LESS THAN 22" X 34"
REDUCED PRINT - USE GRAPHIC SCALES

EMBEDMENT AND LAP NOTES

ALL CONTINUOUS REINFORCEMENT SHALL BE PLACED USING MINIMUM LAP SPLICE LENGTHS AS SHOWN ON TABLE

PROVIDE STANDARD AC318 90 OR 180 DEGREE HOOK IF EMBEDMENT IS LESS THAN MINIMUM SHOWN IN TABLE.

REINFORCEMENT PASSING BETWEEN ELEMENTS (CMU/CONCRETE, SLAB/WALL, BEAM/COLUMN etc.) SHALL HAVE MINIMUM EMBEDMENT ON EACH SIDE OF INTERFACE

FOR SLABS, LOCATED BOTTOM BAR SPLICES ABOVE SUPPORTS, TOP BAR SPLICES AT MID THIRD OF SPAN, STAGGER ALL SPLICES BY NO LESS THAN THE REQUIRED MIN SPLICE LENGTH

FOR DOUBLE CURTAIN WALL REINFORCEMENT STAGGER SPLICES OF TWO CURTAINS BY NO LESS THAN REQUIRED MIN SPLICE LENGTH

FOR COLUMNS LOCATE VERTICAL BAR SPLICES AT MIDDLE OF CLEAR HEIGHT BETWEEN STORIES

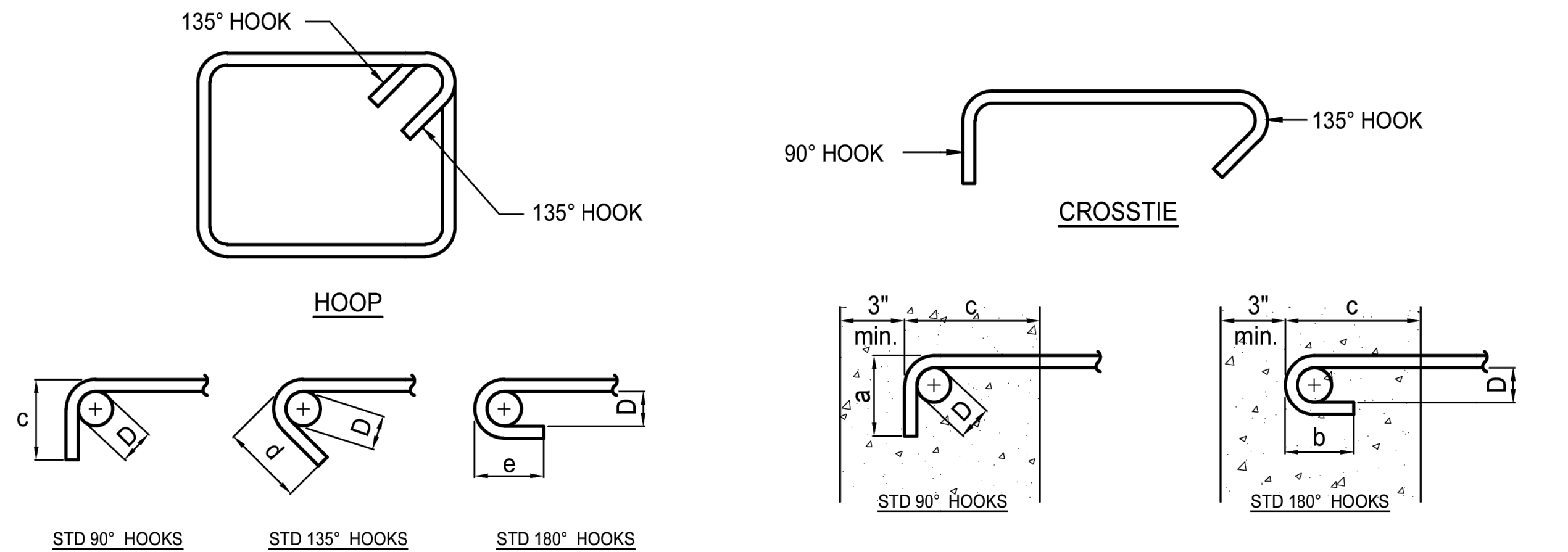
FOR BEAMS LOCATE TOP BAR SPLICES AT MIDSPAN, BOTTOM BAR SPLICES AT THIRD POINTS OF SPAN.

FOR BEAMS AND COLUMNS DO NOT PLACE ANY SPLICES IN BEAM-COLUMN JOINTS

MINIMUM DEVELOPMENT AND SPLICE LENGTHS FOR CMU AND CONCRETE				
BAR SIZE	MINIMUM STRAIGHT EMBEDMENT (INCHES)		MINIMUM LAP REQUIREMENTS (INCHES)	
	TOP *	OTHERS	TOP *	OTHERS
#3	22	16	28	22
#4	28	21	36	28
#5	36	27	46	36
#6	42	32	54	42
#7	48	38	64	48
#8	56	45	72	56
#9	64	49	100	64

* - TOP BARS HAVE MORE THAN 12 INCHES OF FRESH CONCRETE CAST BELOW THEM

BENDS, HOOPS & CROSSTIES



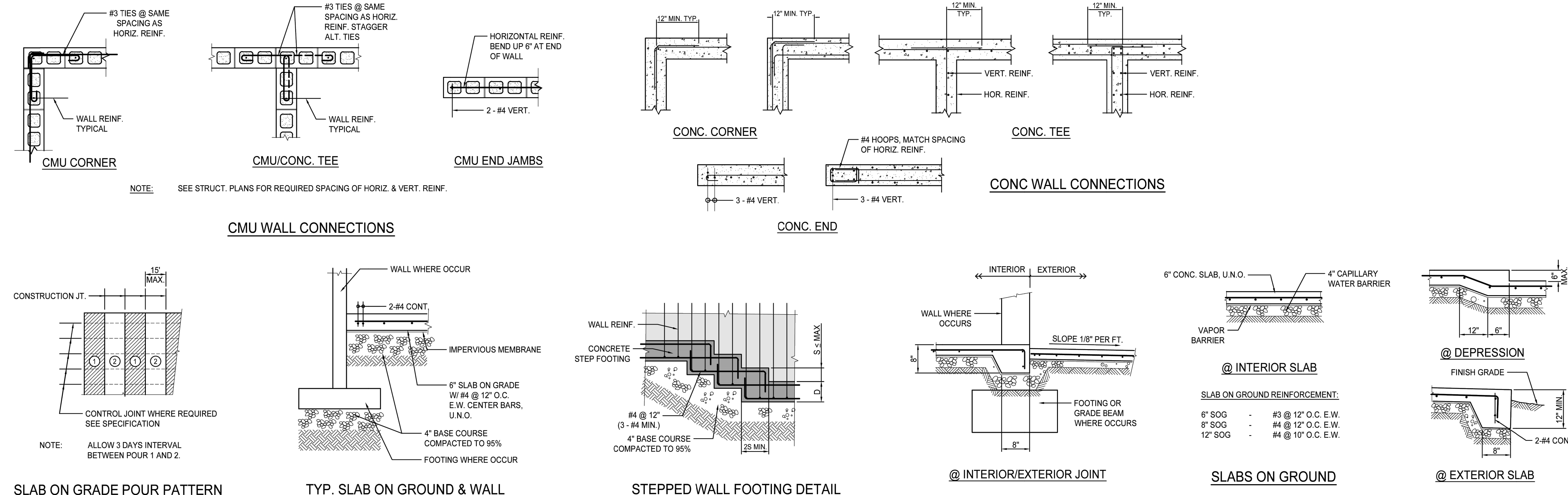
HOOPS & CROSSTIES - STD 90°, 135° & 180° HOOKS				
BAR SIZE	c (MIN)	d (MIN)	e (MIN)	D
#3	5"	5"	5"	1 1/2"
#4	5"	5"	5"	2"
#5	6"	6"	6"	2 1/2"
#6	12"	8"	8"	4 1/2"

PRIMARY REINFORCING STD 90° and 180° HOOK				
BAR SIZE	a (MIN.)	b (MIN.)	c (MIN.)	D
#3	6"	6"	9"	2 1/4"
#4	8"	6"	12"	3"
#5	10"	6"	14"	3 3/4"
#6	12"	6"	18"	4 1/2"
#7	14"	8"	20"	5 1/4"
#8	16"	8"	22"	6"
#9	19"	15"	25"	9"
#10	21"	17"	28"	10"

STRUCTURAL ABBREVIATIONS

BOT.	BOTTOM	INFO.	INFORMATION
C.I.P.	CAST-IN-PLACE	INT.	INTERIOR
C.J.	CONSTRUCTION JOINT	KSF	KIPS PER SQUARE FEET
CL	CENTERLINE	KSI	KIPS PER SQUARE INCH
CLR.	CLEAR(ANCE)	MAX.	MAXIMUM
CMU	CONCRETE MASONRY UNIT	MFR	MANUFACTURER
CONC.	CONCRETE	MIN.	MINIMUM
CONT.	CONTINUOUS	MISC.	MISCELLANEOUS
DET.	DETAIL	N.T.S.	NOT TO SCALE
DIA.	DIAMETER	O.C.	ON CENTER
DIM.	DIMENSION	O.D.	OUTSIDE DIAMETER
DWG.	DRAWING	OPP.	OPPOSITE
E.A.	EACH	PL	PLATE
E.F.	EACH FACE	PSI	POUNDS PER SQUARE INCH
E.J.	EXPANSION JOINT	PSF	POUNDS PER SQUARE FEET
EQ.	EQUAL	REINF.	REINFORCED, REINFORCEMENT
EQUIPT.	EQUIPMENT	SHT.	SHEET
E.S.	EACH SIDE	S.O.G.	SLAB ON GRADE
E.W.	EACH WAY	SPECS.	SPECIFICATION
EXP	EXPANSION	SQ.	SQUARE
EXIST.	EXISTING	STRUCT.	STRUCTURAL
EXT.	EXTERIOR	T	THICKNESS
FDN	FOUNDATION	T&B	TOP & BOTTOM
FFE	FINISH FLOOR ELEVATION	TM	TOP MOST BARS
FL	FLOOR	BM	BOTTOM MOST BARS
F.O.C.	FACE OF CONCRETE	T.O.F.	TOP OF FOOTING
FT.	FOOT OR FEET	T.O.S.	TOP OF SLAB, TOP OF STEEL
FTG.	FOOTING	T.O.W.	TOP OF WALL
GA.	GAUGE	TYP.	TYPICAL
GALV.	GALVANIZED	U.N.O.	UNLESS NOTED OTHERWISE
HORIZ.	HORIZONTAL	VERT.	VERTICAL
I.D.	INSIDE DIAMETER	W/	WITH

TYPICAL DETAILS

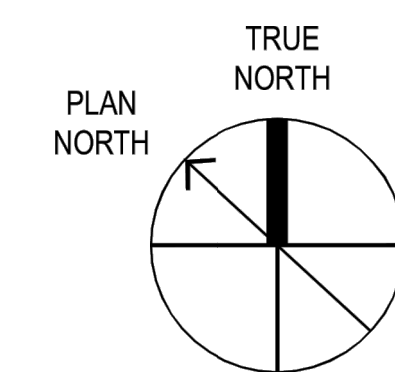
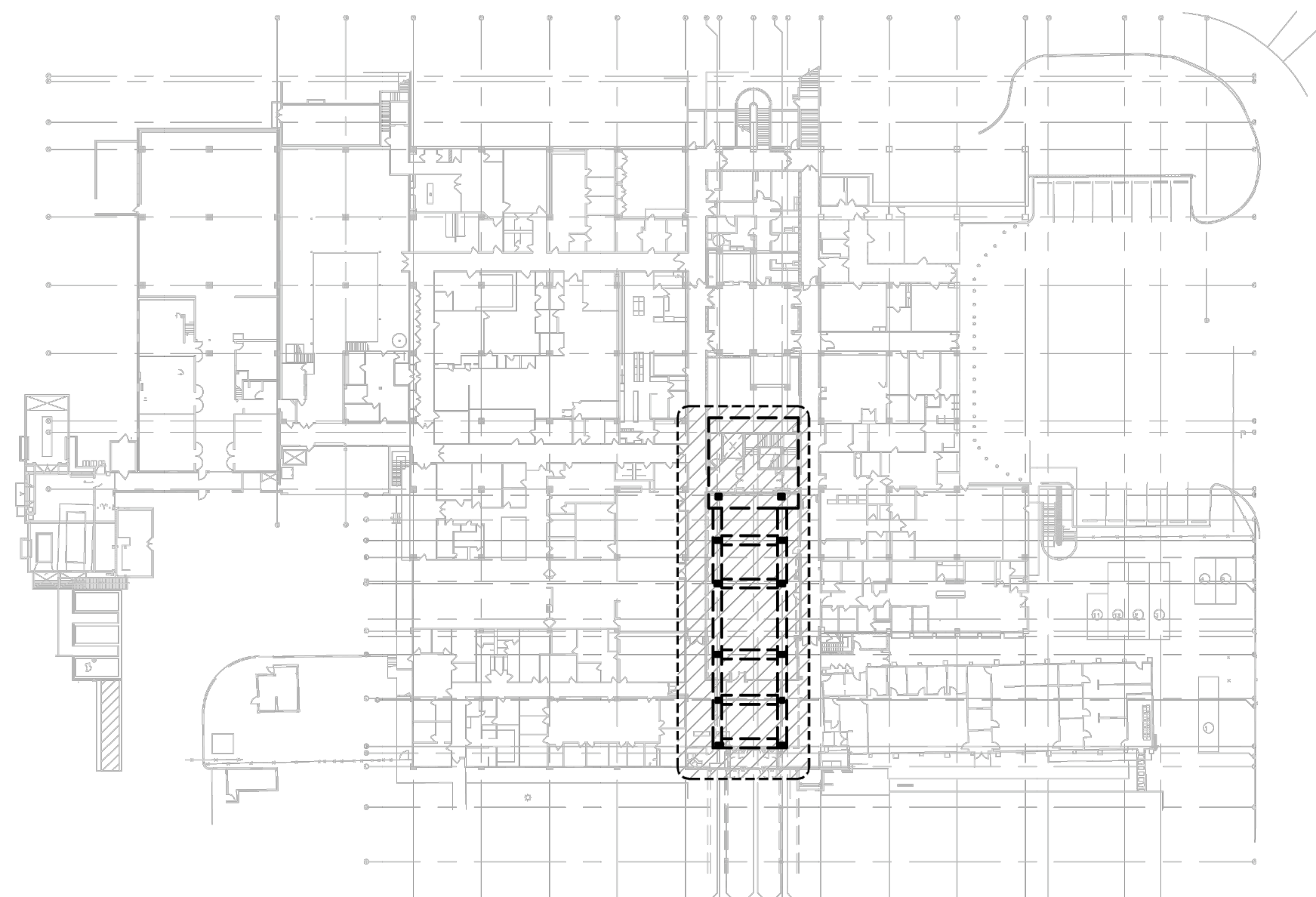


IF SHEET IS LESS THAN 22" X 34"
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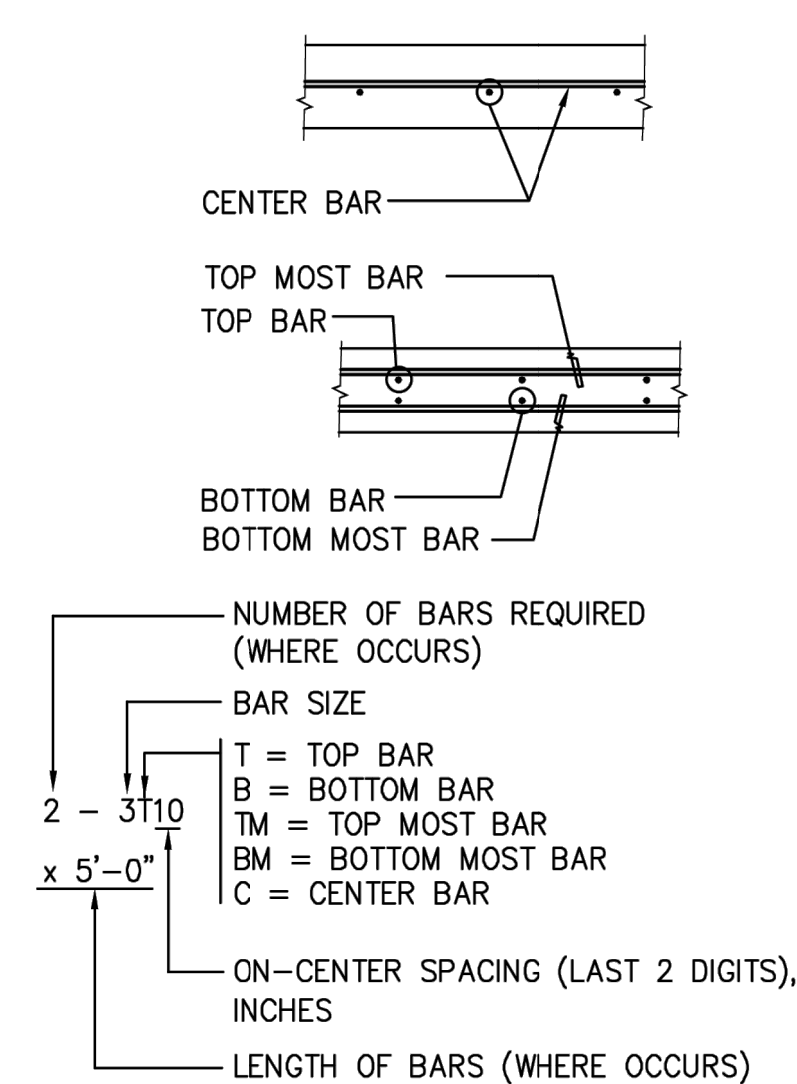


**MCH RENOVATION PROJECT, GMHA 007-2014
GMHA FAMILY BIRTH CENTER**
850 GOVERNOR CAMACHO ROAD, OKA, TAMUNING, GUAM 96913
GUAM MEMORIAL HOSPITAL AUTHORITY
STATEMENT OF SPECIAL INSPECTIONS

[illegible]

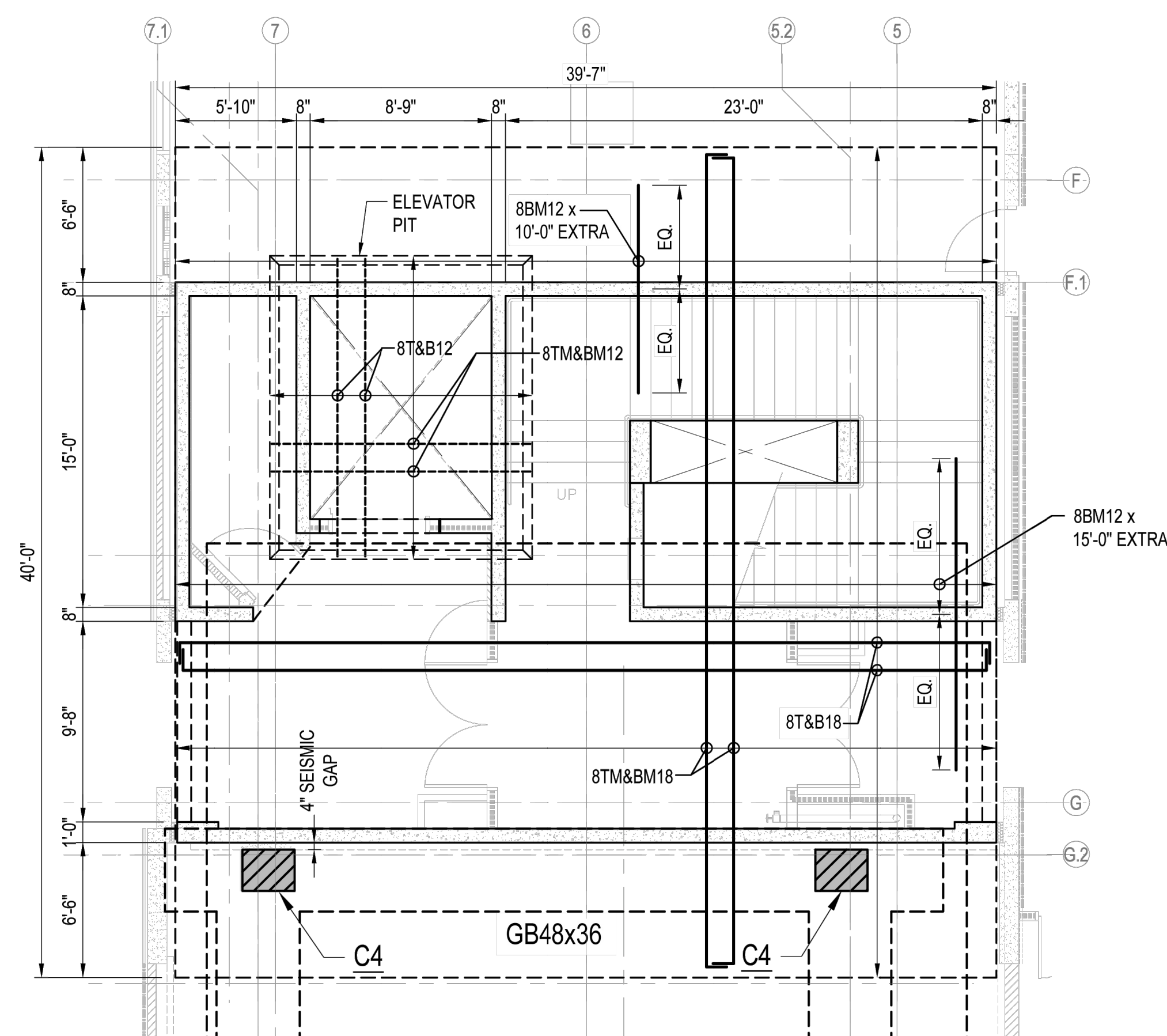


KEYPLAN



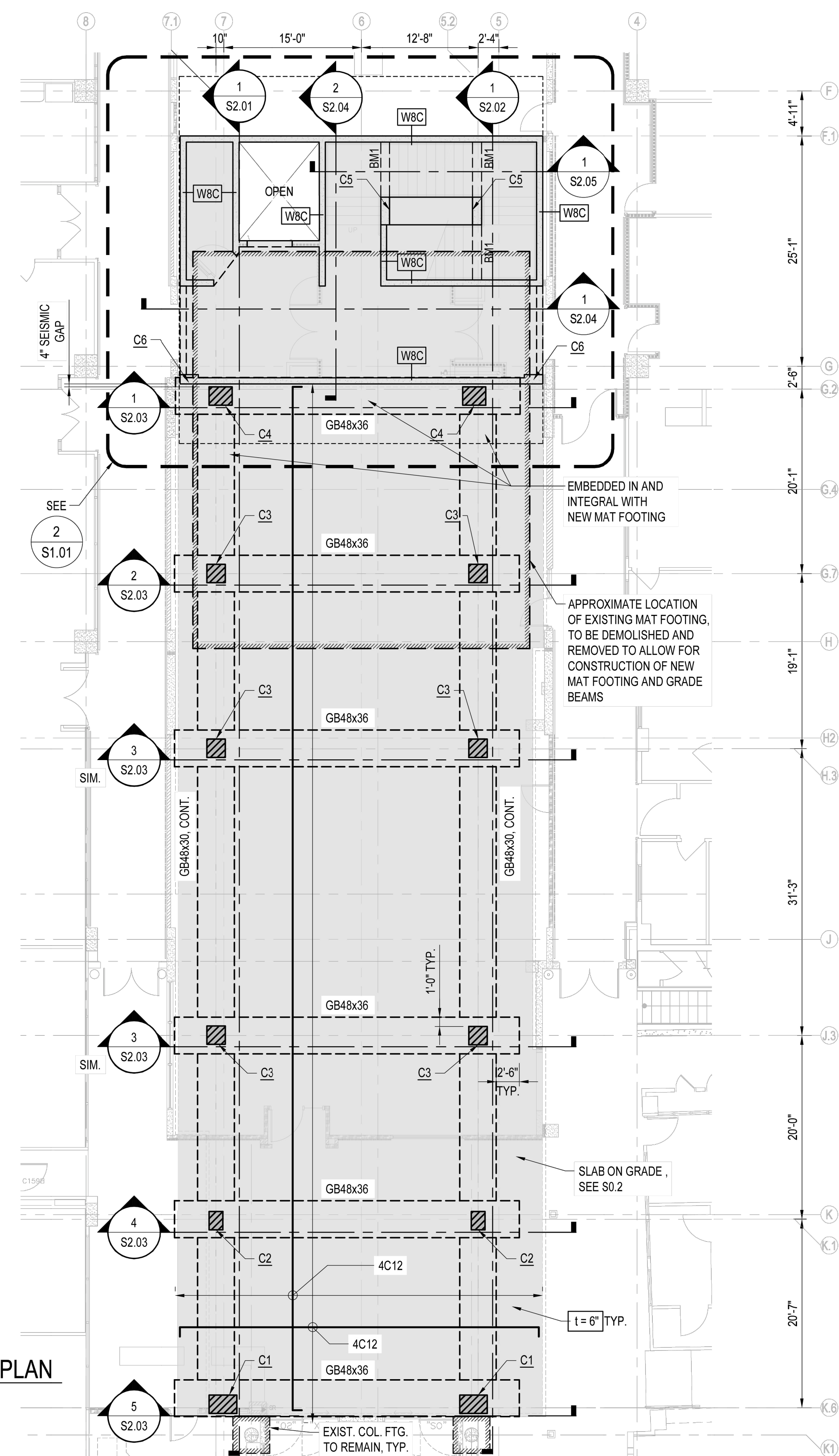
SLAB REINF. DETAIL

- LEGEND:
- GBxx ----- GRADE BEAM DESIGNATION, SEE SHEET S2.01
 - CL ----- COLUMN DESIGNATION, SEE SHEET S3.01
 - WBC] ----- WALL DESIGNATION, SEE SHEET S3.01

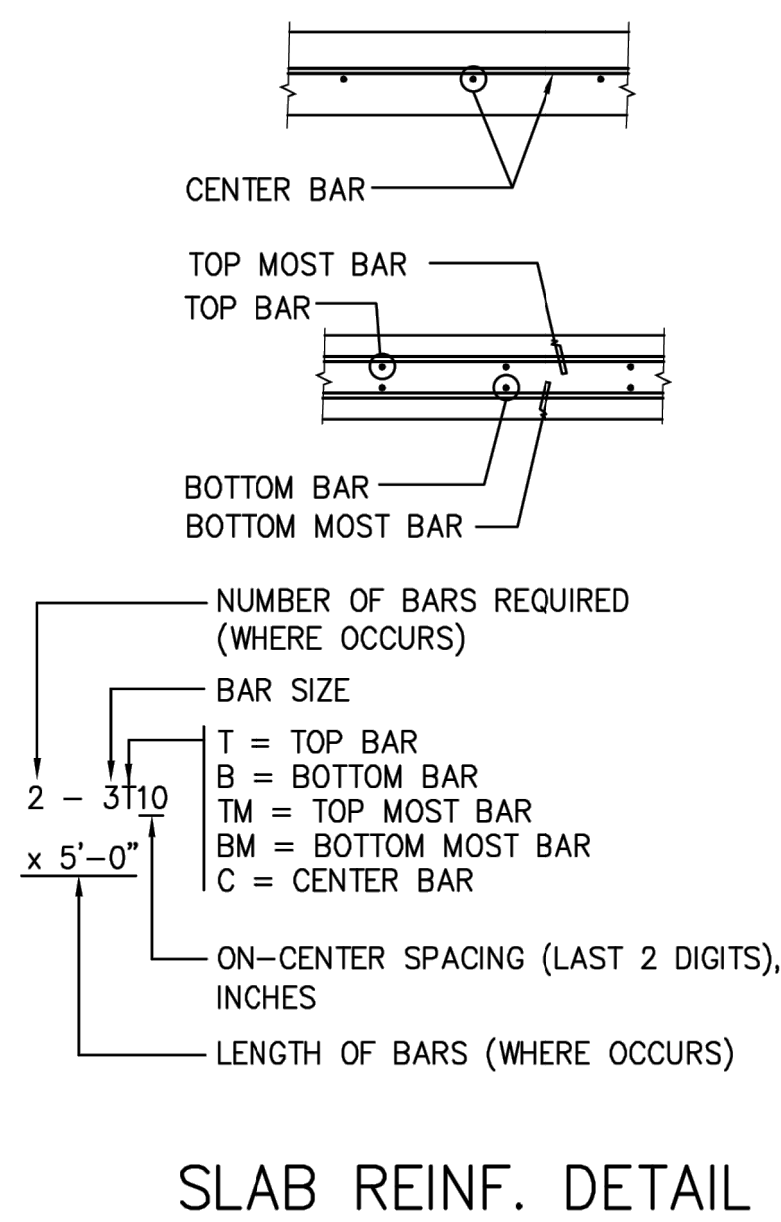
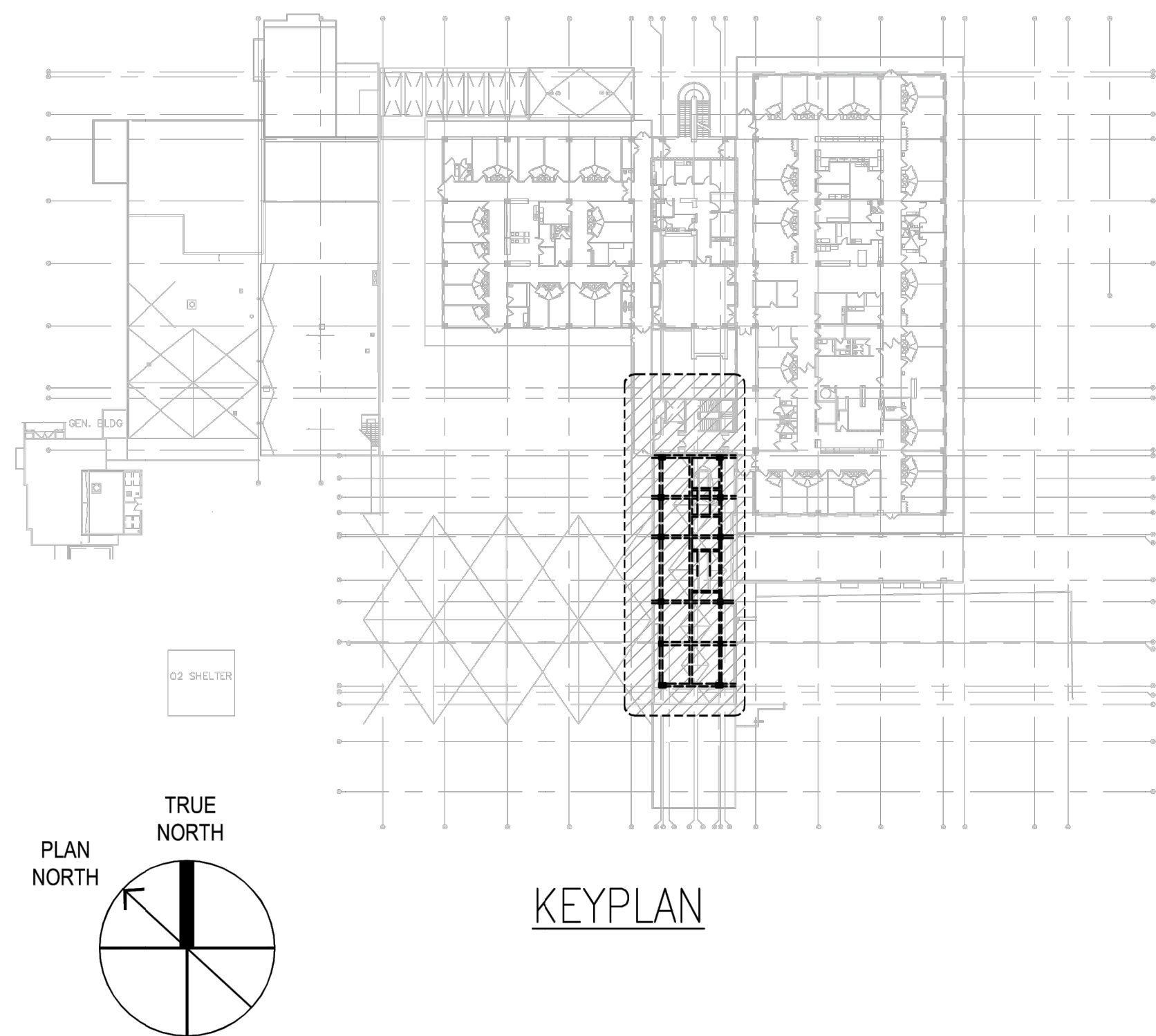


2 MAT SLAB ENLARGED PLAN
SCALE 3/16" = 1'-0"

1 FOUNDATION PLAN
SCALE 1/8" = 1'-0"

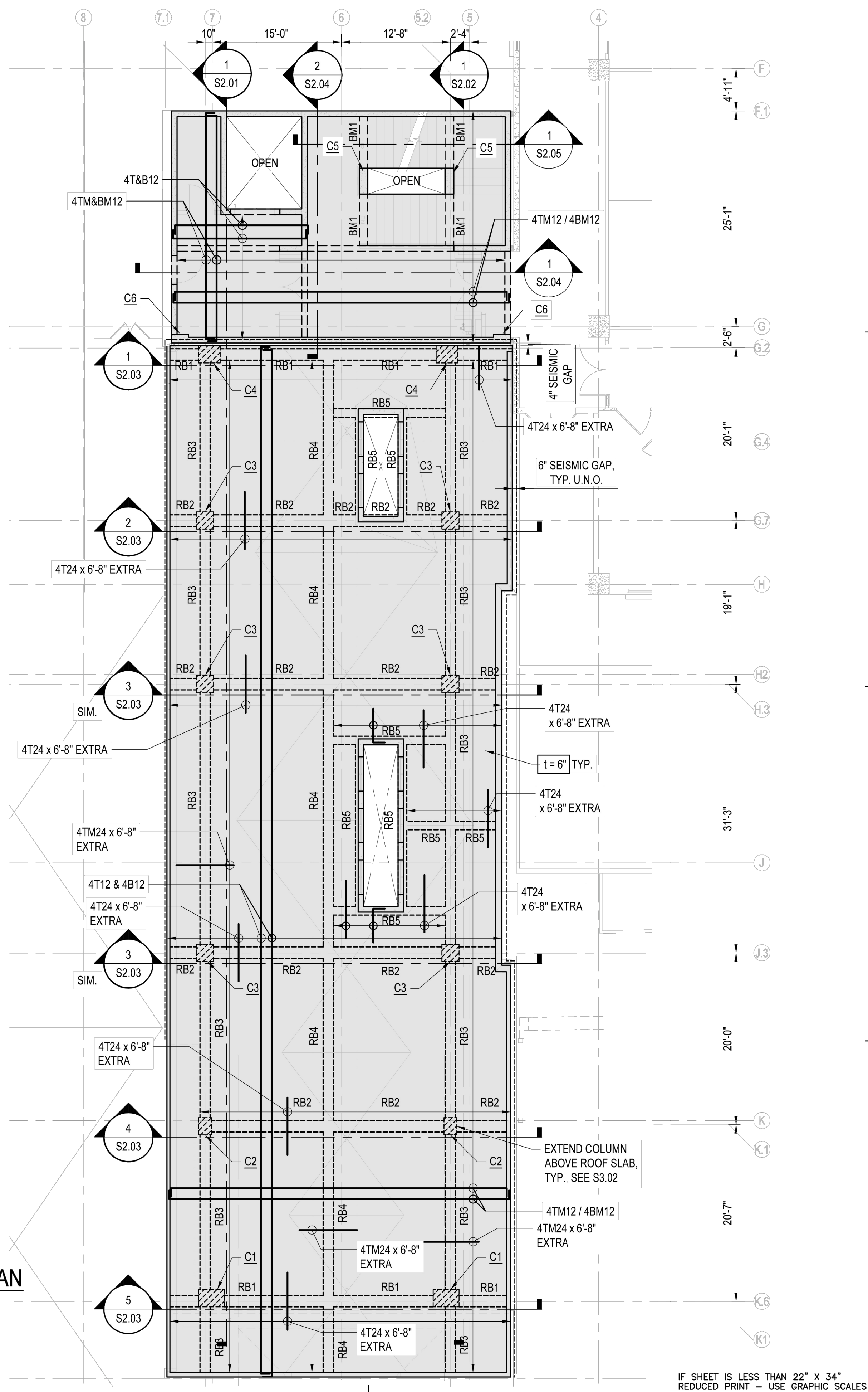


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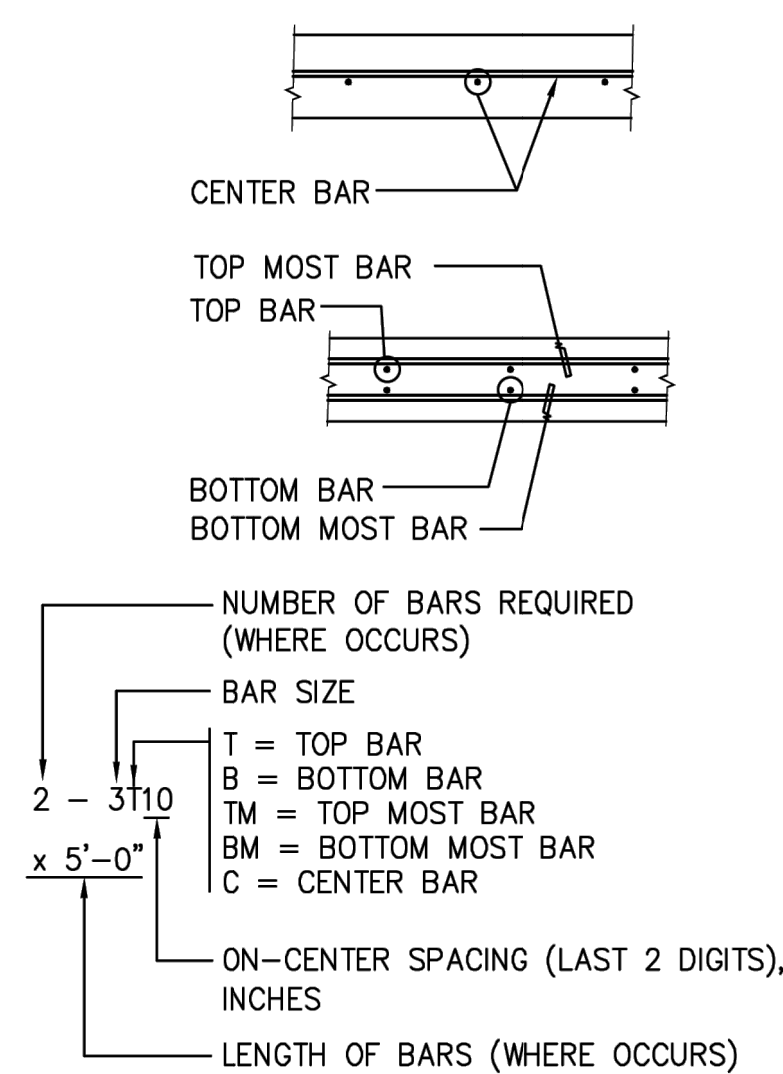
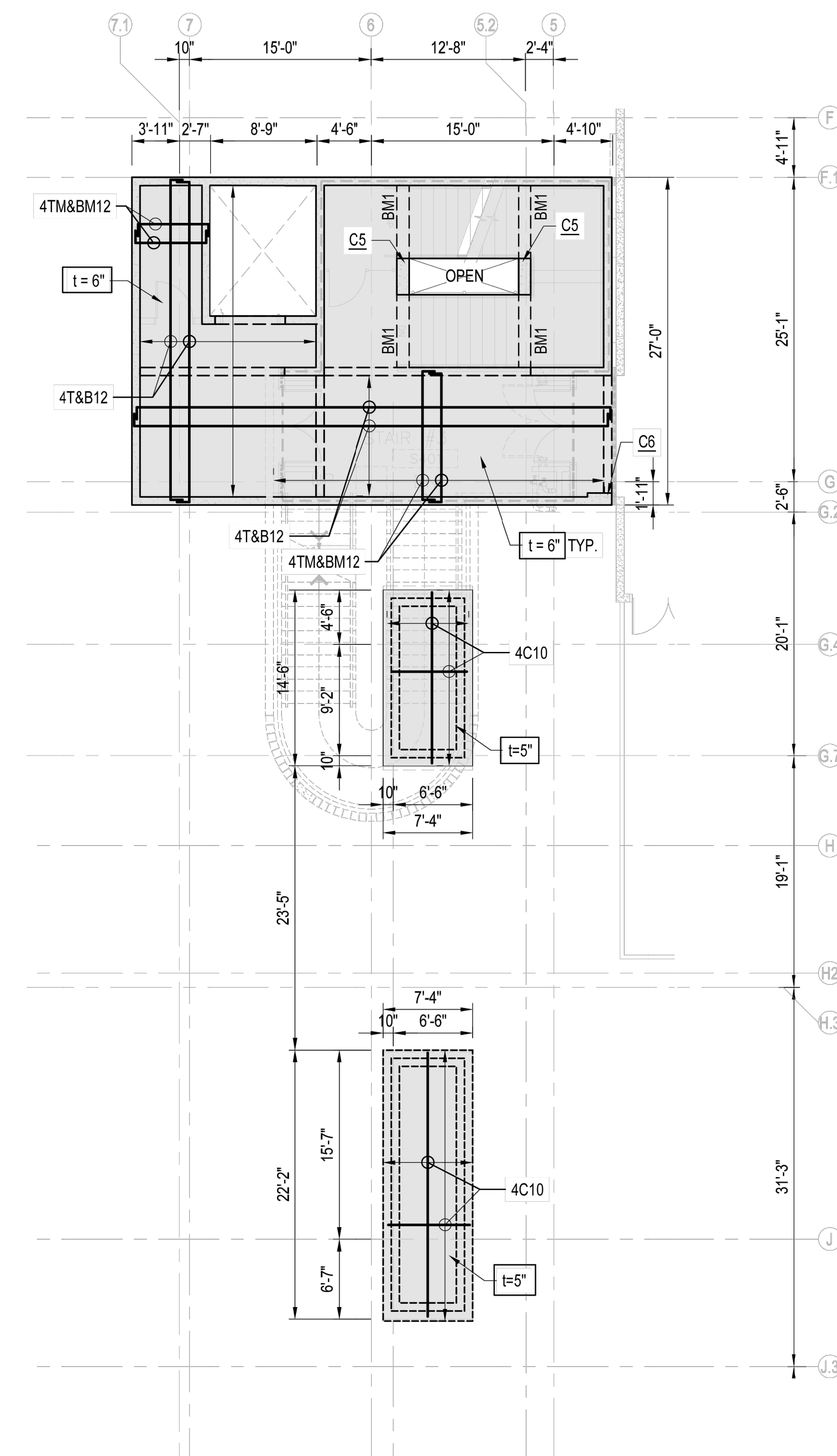
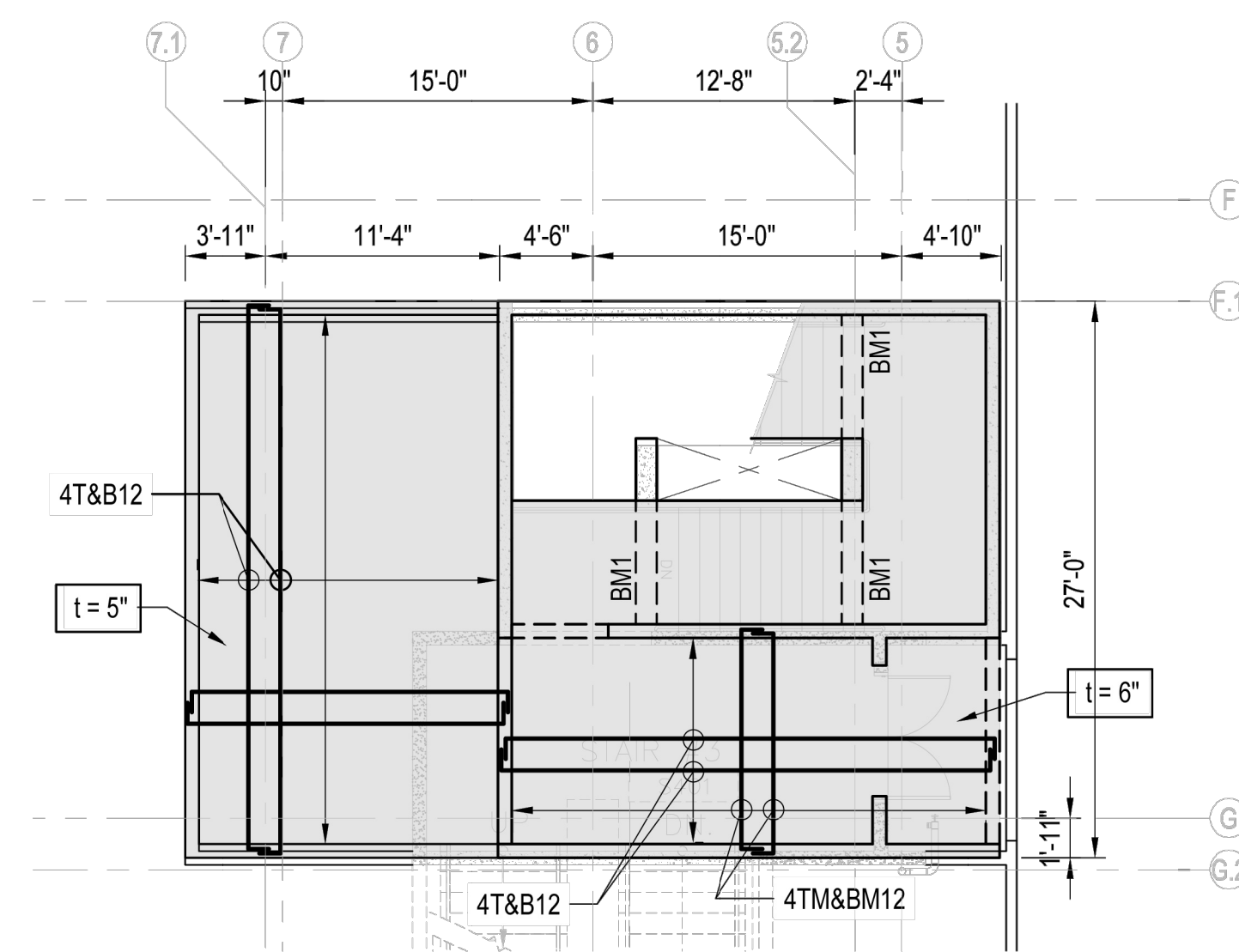
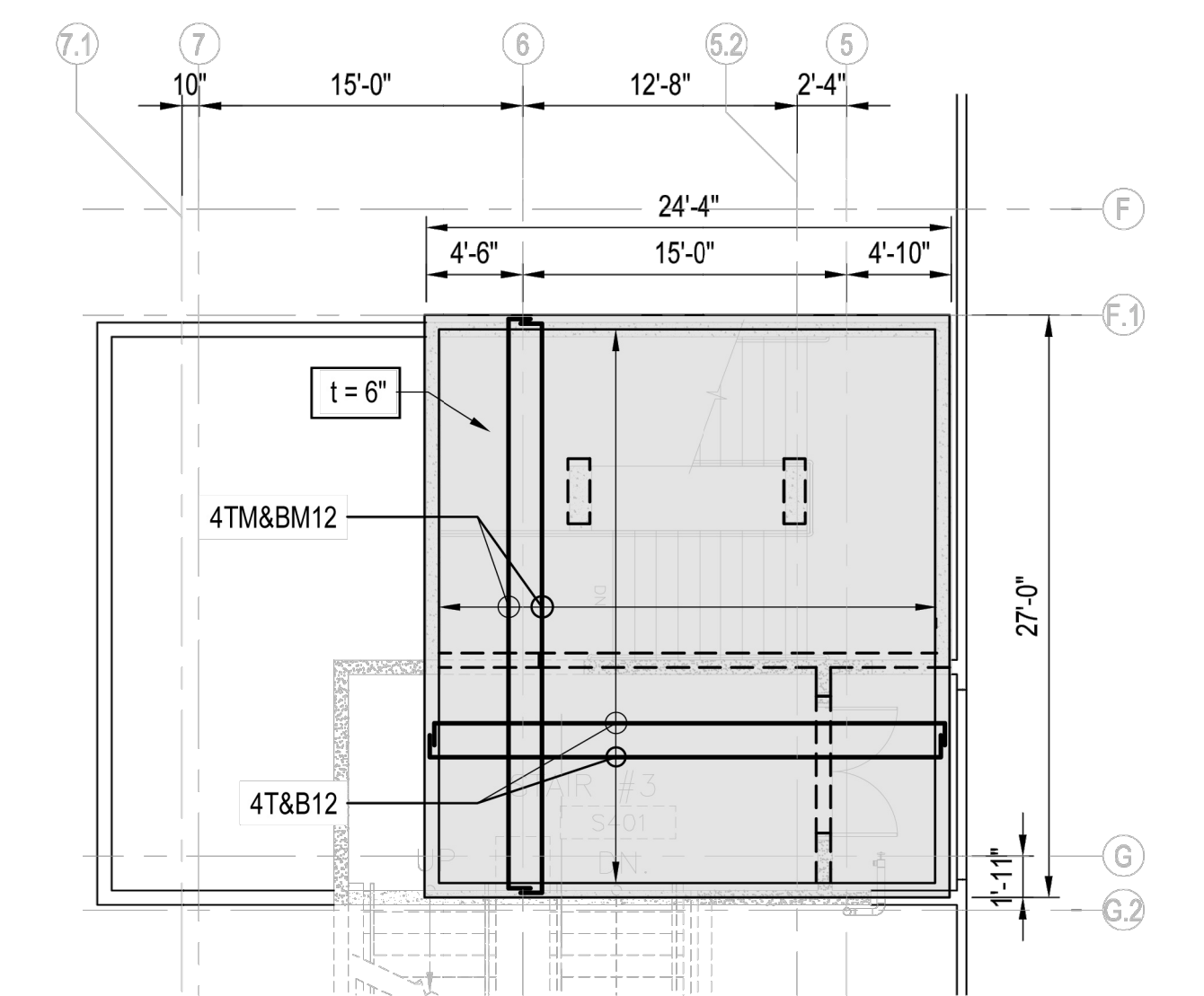


LEGEND:
C1 ----- COLUMN DESIGNATION, SEE SHEET S3.01
B1 ----- BEAM DESIGNATION, SEE SHEET S3.01
W8C ----- WALL DESIGNATION, SEE SHEET S3.01
t=6" ----- SLAB THICKNESS

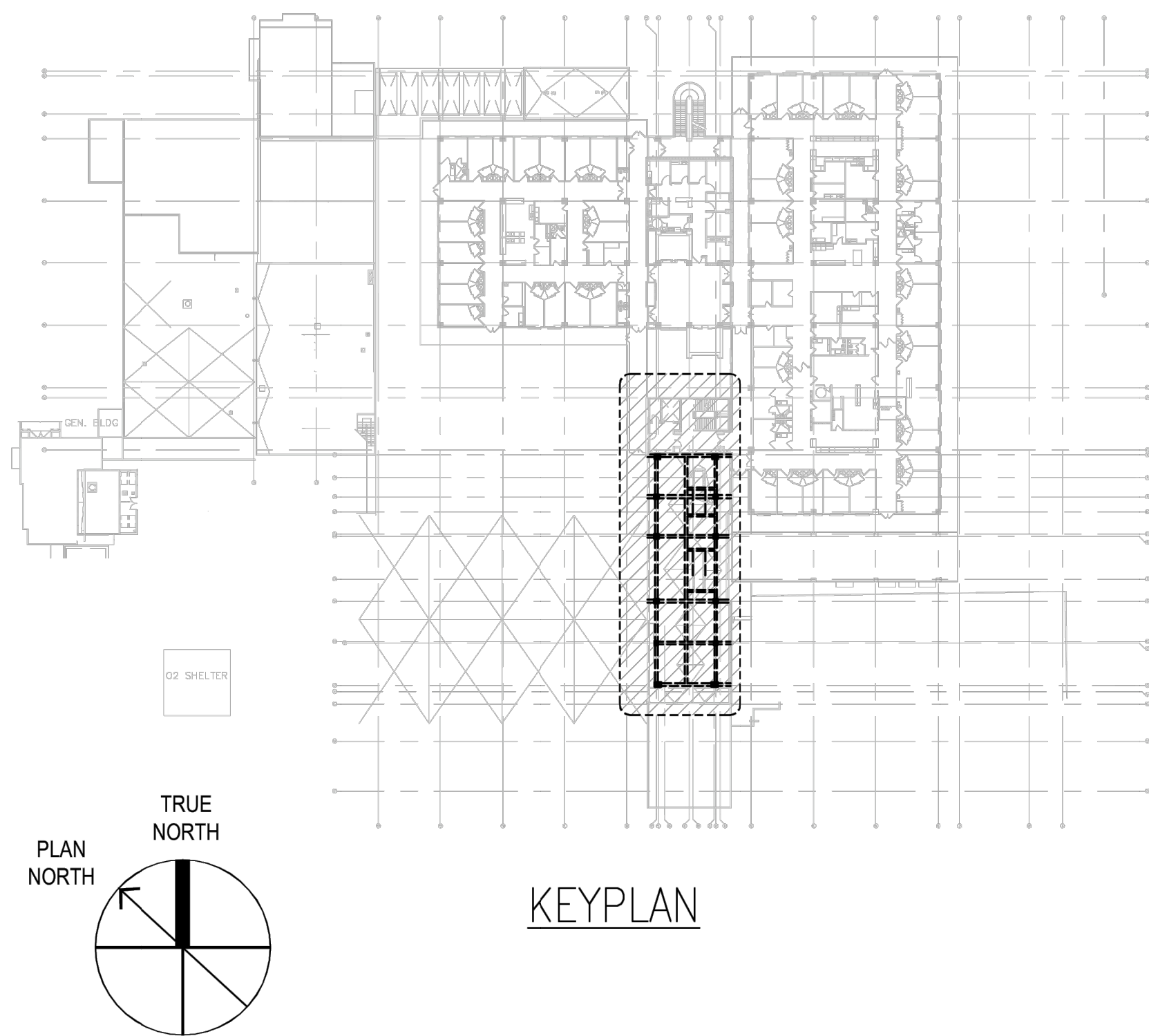
3RD LEVEL / ROOF FRAMING PLAN
SCALE 1/8" = 1'-0"



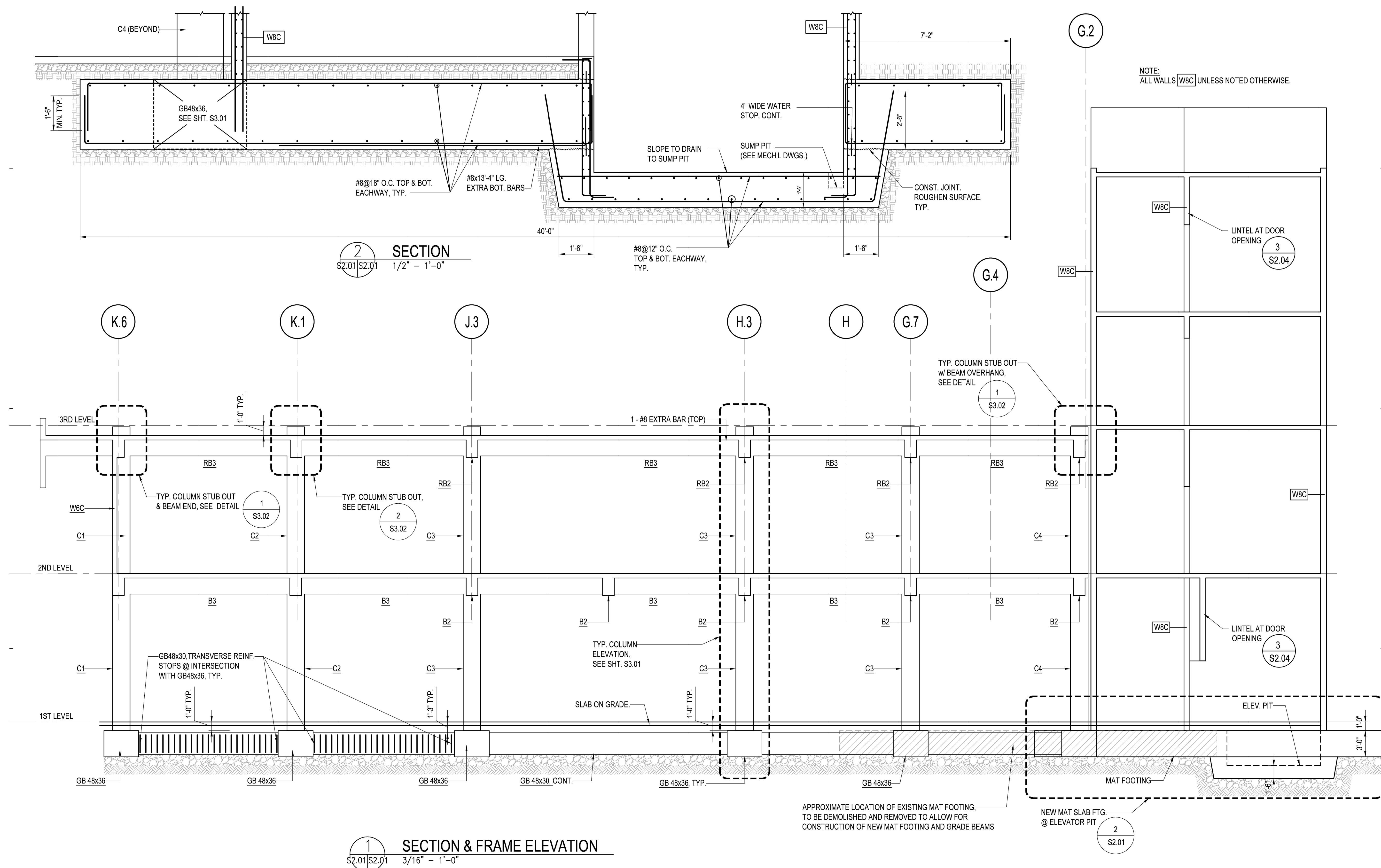
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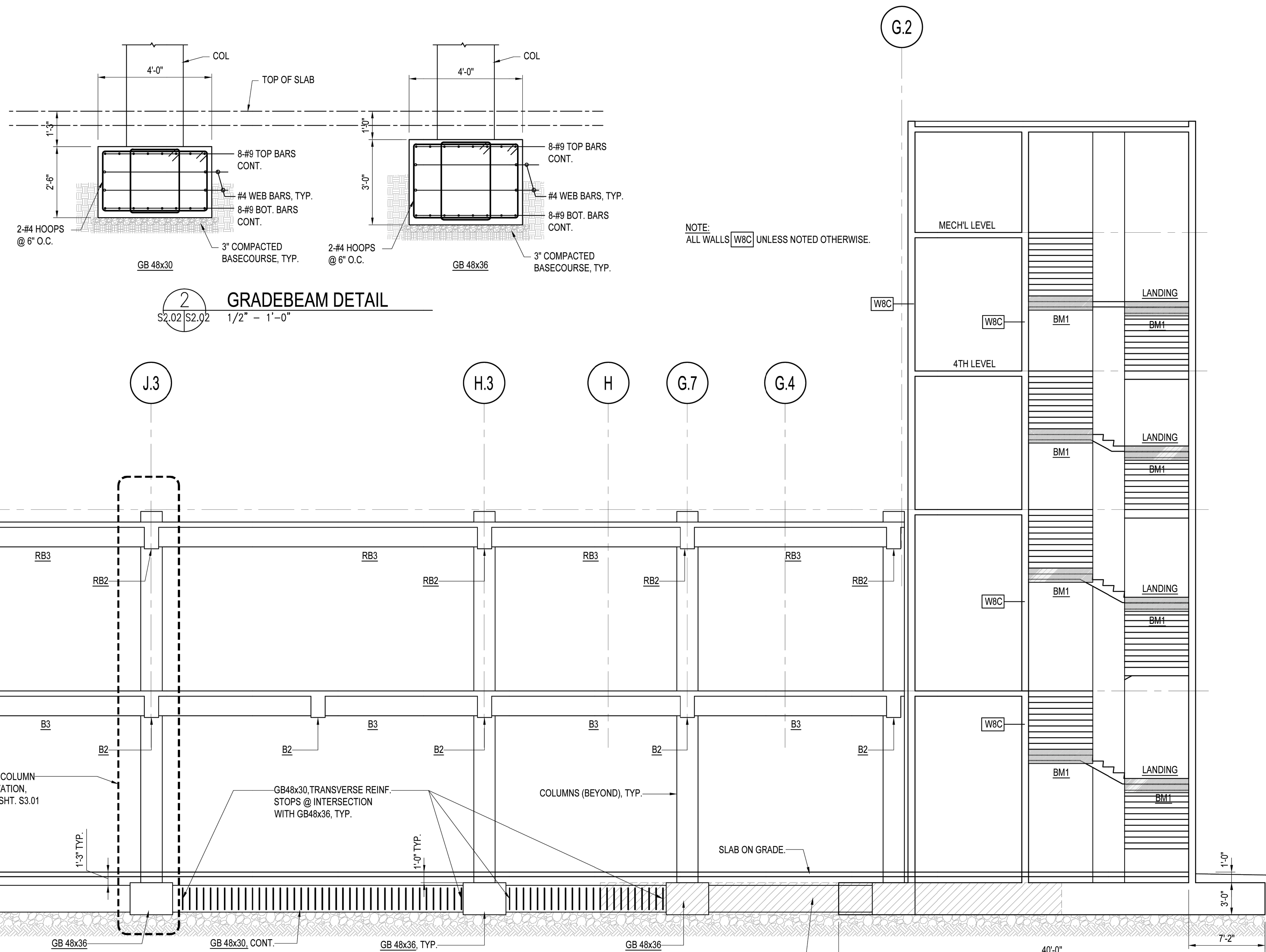


- LEGEND:
- C1 - COLUMN DESIGNATION, SEE SHEET S3.01
 - B1 - BEAM DESIGNATION, SEE SHEET S3.01
 - WBC - WALL DESIGNATION, SEE SHEET S3.01
 - t=6" - SLAB THICKNESS



IF SHEET IS LESS THAN 22" X 34"
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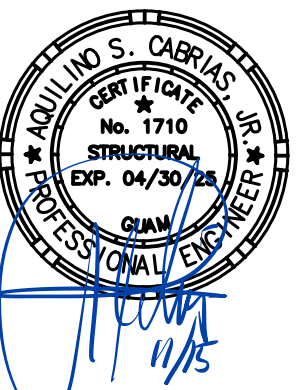




SECTION & FRAME ELEVATION
S2.02 | S2.02 | 3/16\" - 1'-0\"

APPROXIMATE LOCATION OF EXISTING MAT FOOTING,
TO BE DEMOLISHED AND REMOVED TO ALLOW FOR
CONSTRUCTION OF NEW MAT FOOTING AND GRADE BEAMS

IF SHEET IS LESS THAN 22\" X 34\"
REDUCED PRINT - USE GRAPHIC SCALES



I CERTIFY THAT THIS WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION

PERMIT SET

MCH RENOVATION PROJECT, GMHA 007-2014
GMHA FAMILY BIRTH CENTER

850 GOVERNOR CAMACHO ROAD, OKA, TAMUNING, GUAM 96913

GUAM MEMORIAL HOSPITAL AUTHORITY

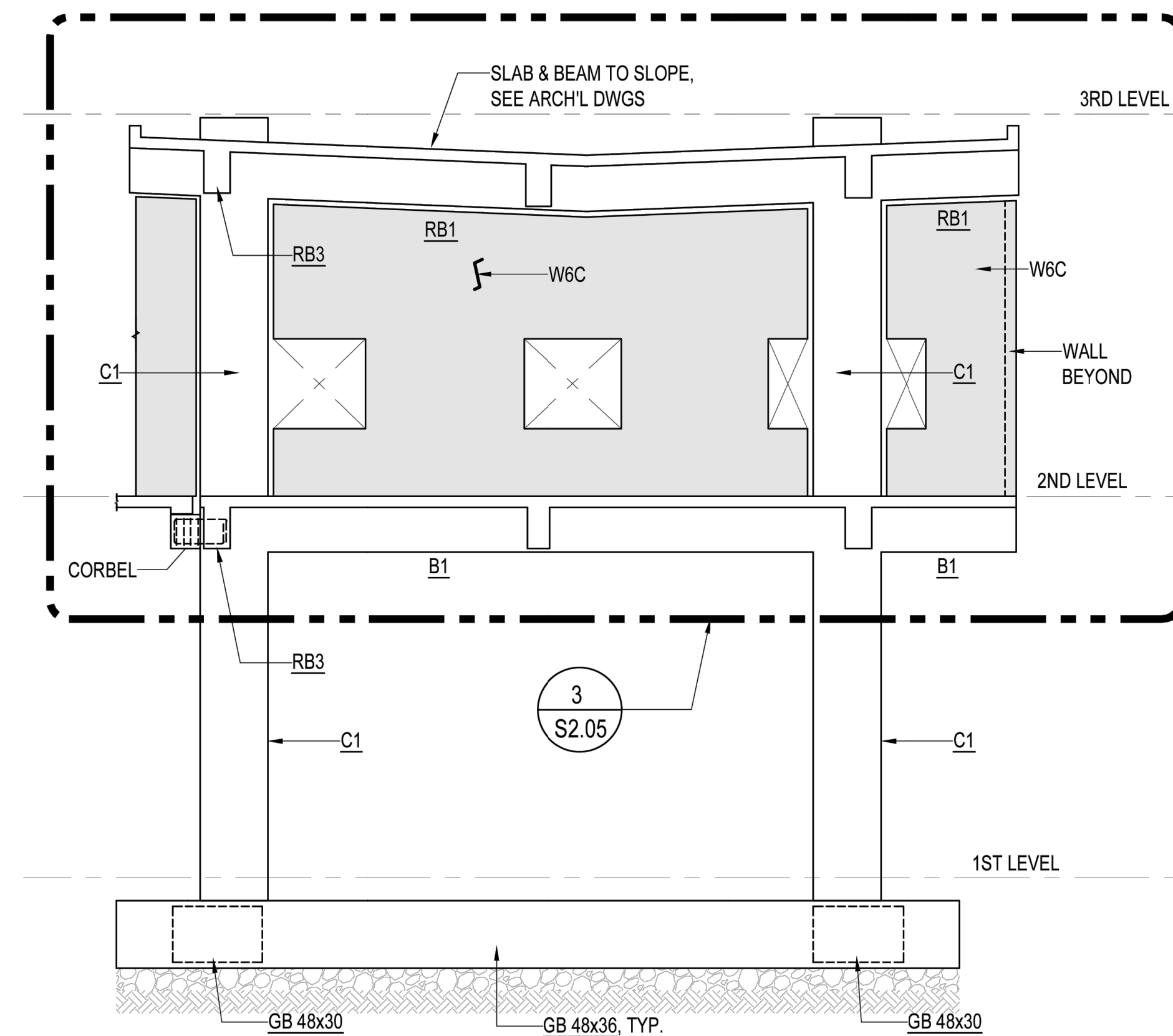
ELEVATIONS

PROJECT TITLE: OWNER: SHEET TITLE:

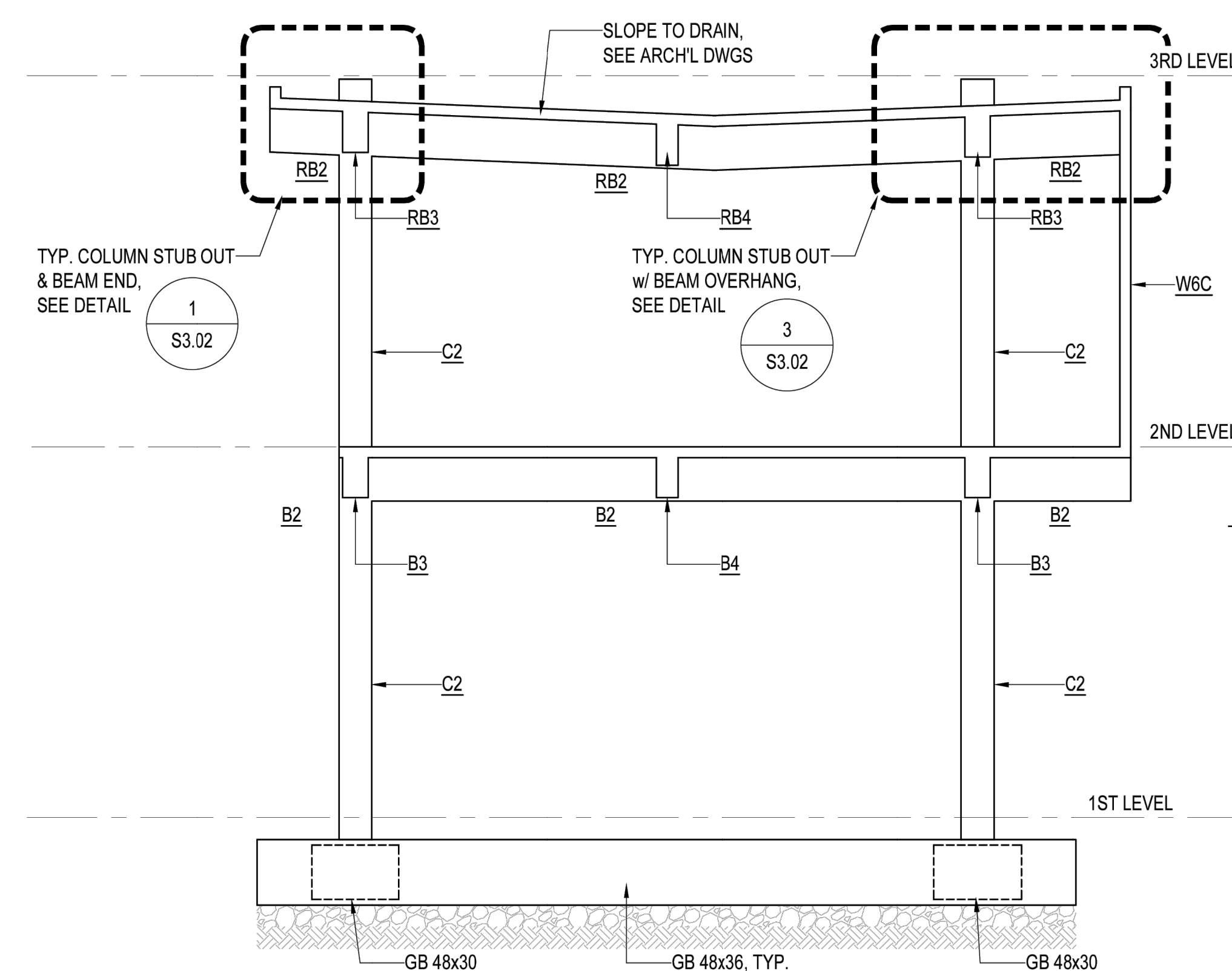
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DATE	2024.11.15	
PROJECT NO	144052	
DRAWN BY	ASC	
CHECKED BY	ASC	
COPYRIGHT		

DWG NO:

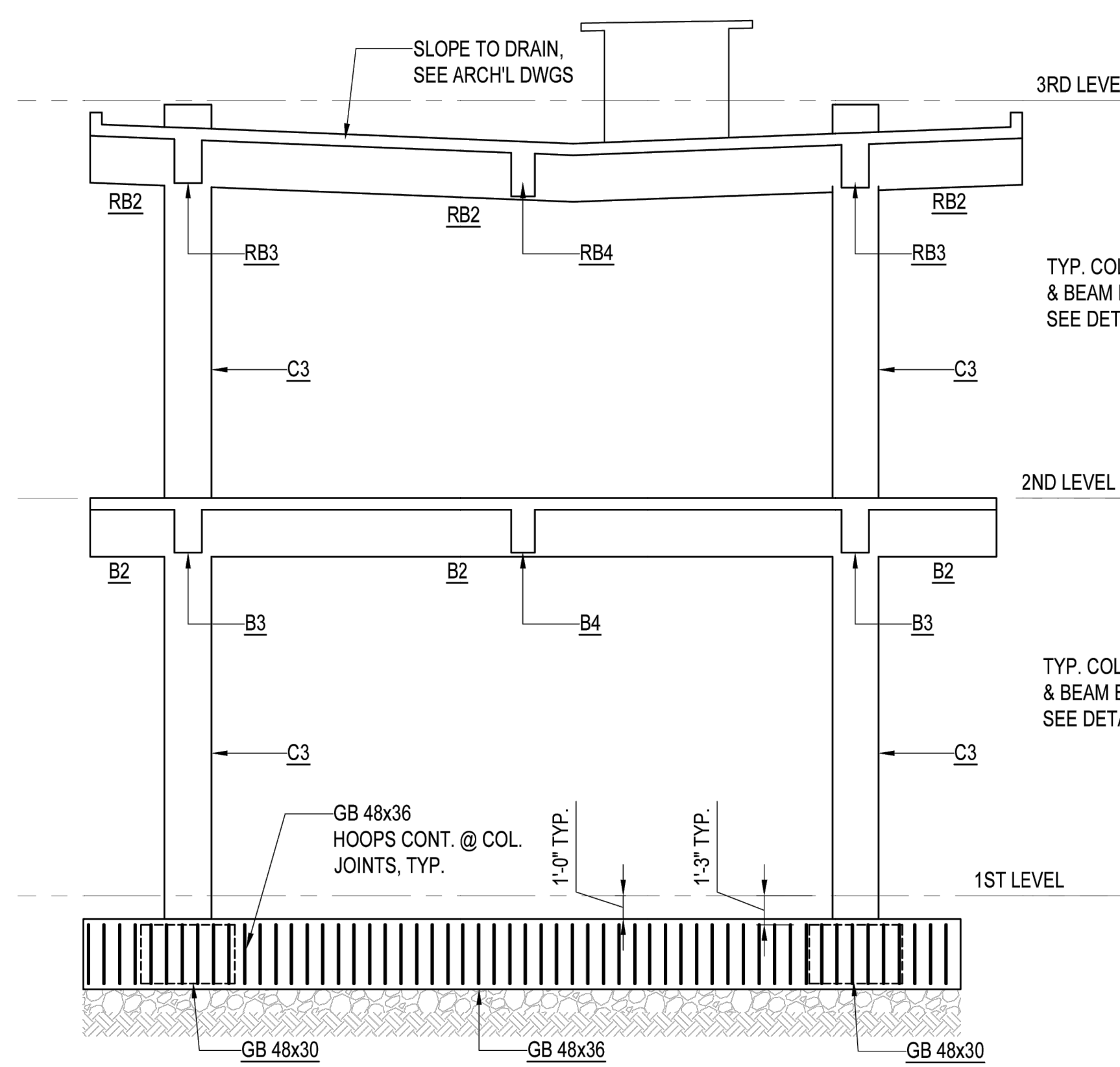
S2.03



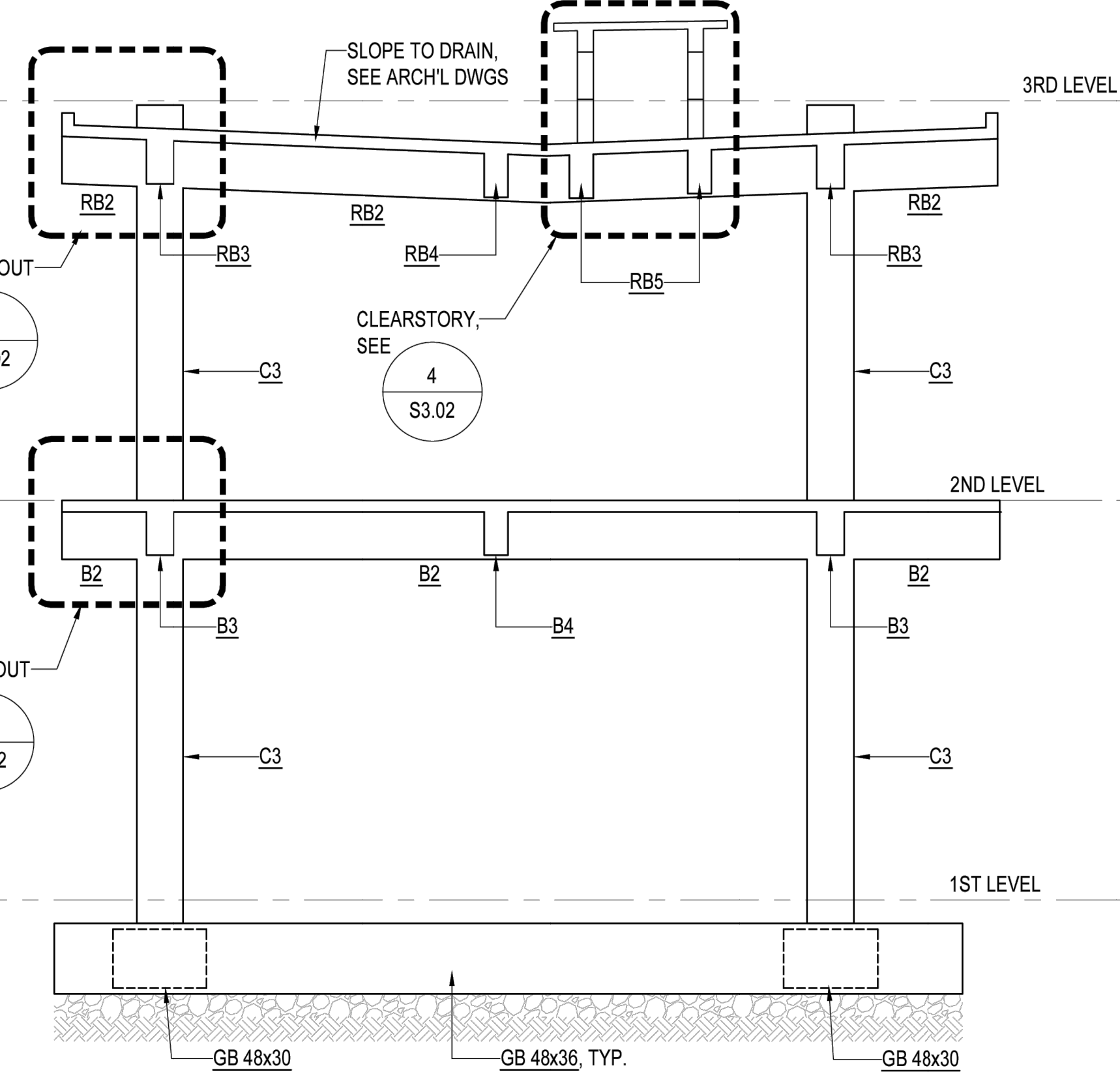
5 ELEVATION
S2.03/S2.03 3/16" - 1'-0"



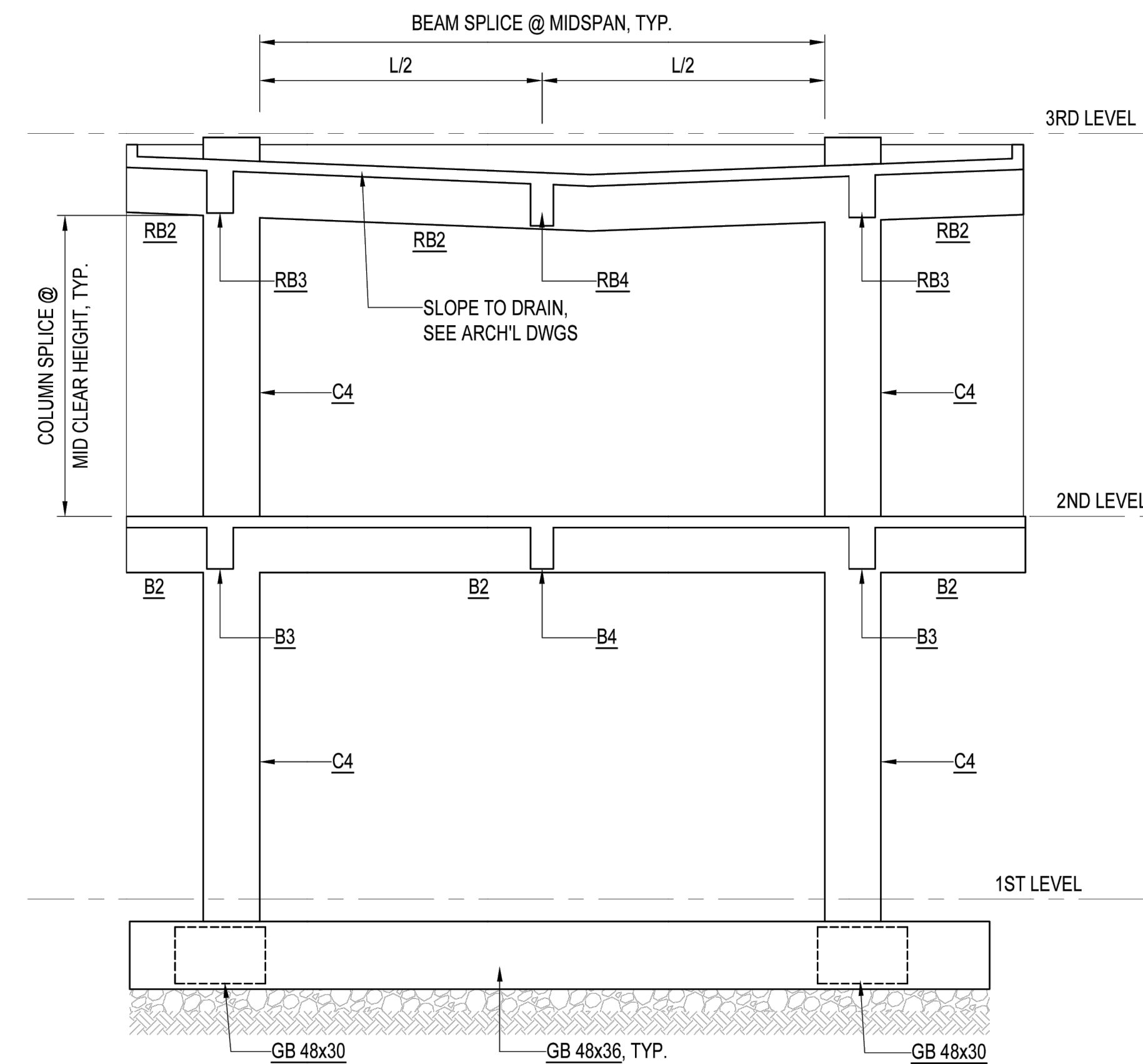
4 ELEVATION
S2.03/S2.03 3/16" - 1'-0"



3 ELEVATION
S2.03/S2.03 3/16" - 1'-0"

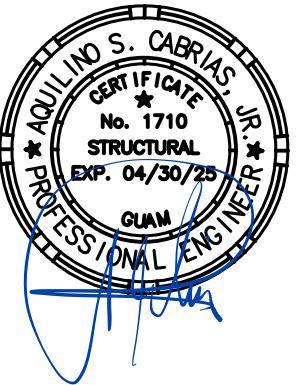


2 ELEVATION
S2.03/S2.03 3/16" - 1'-0"



1 ELEVATION
S2.03/S2.03 3/16" - 1'-0"

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SECTIONS

PROJECT
TITLE:

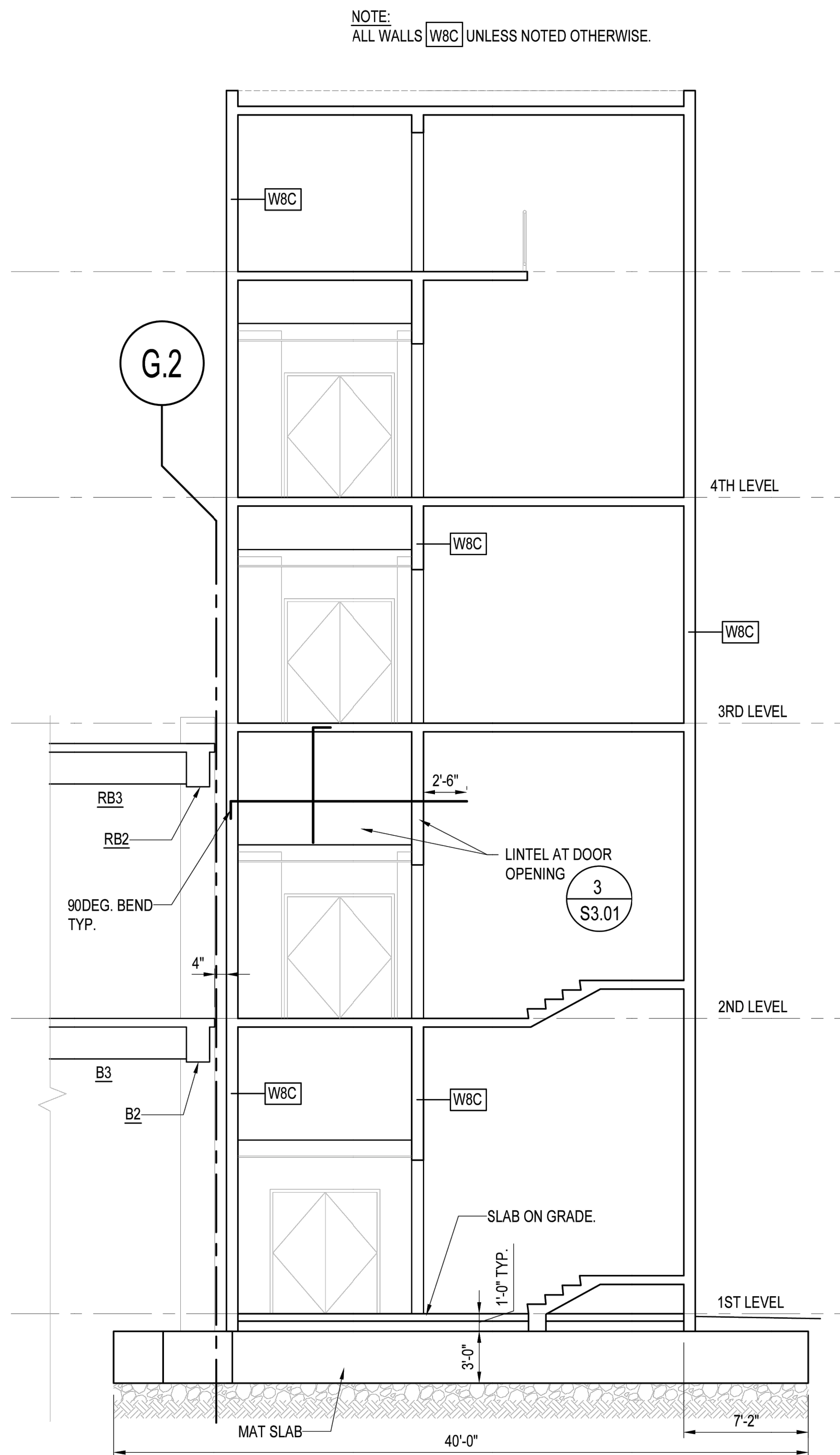
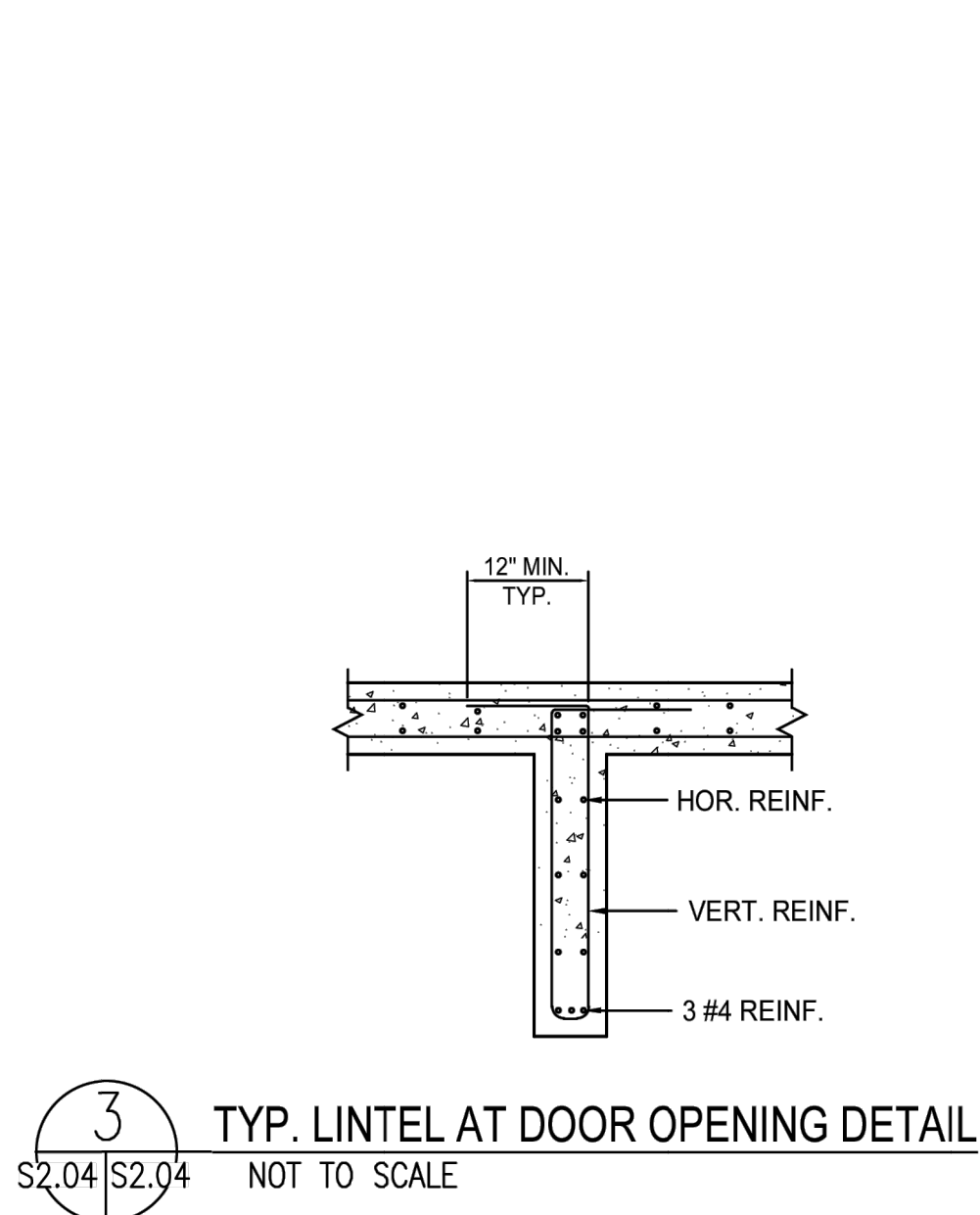
OWNER:

SHEET
TITLE:

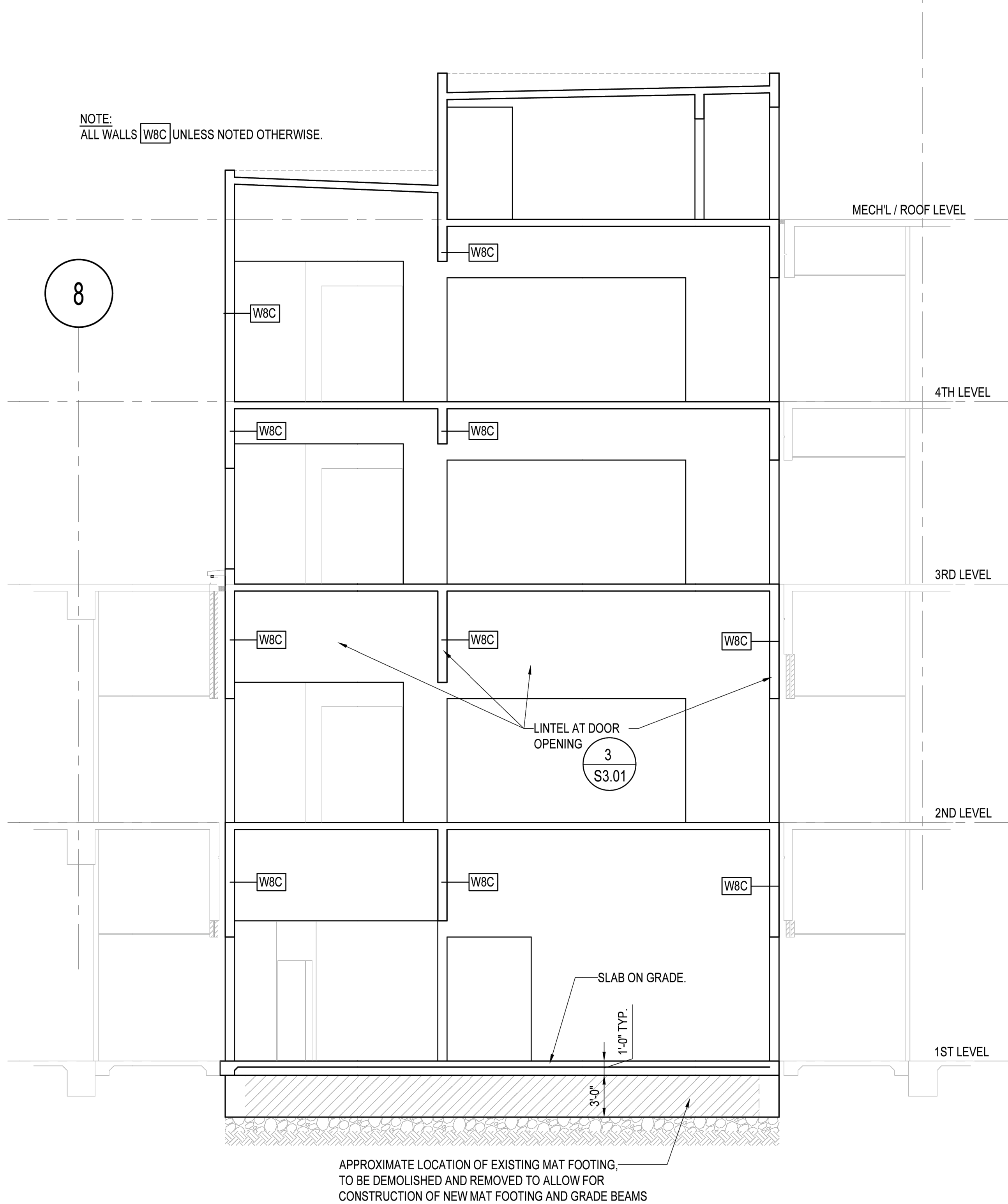
MARK	DATE	DESCRIPTION
DATE	: 2024.11.15	
PROJECT NO	: 144052	
DRAWN BY	: ASC	
CHECKED BY	: ASC	
COPYRIGHT	:	

DWG NO:

S2.04

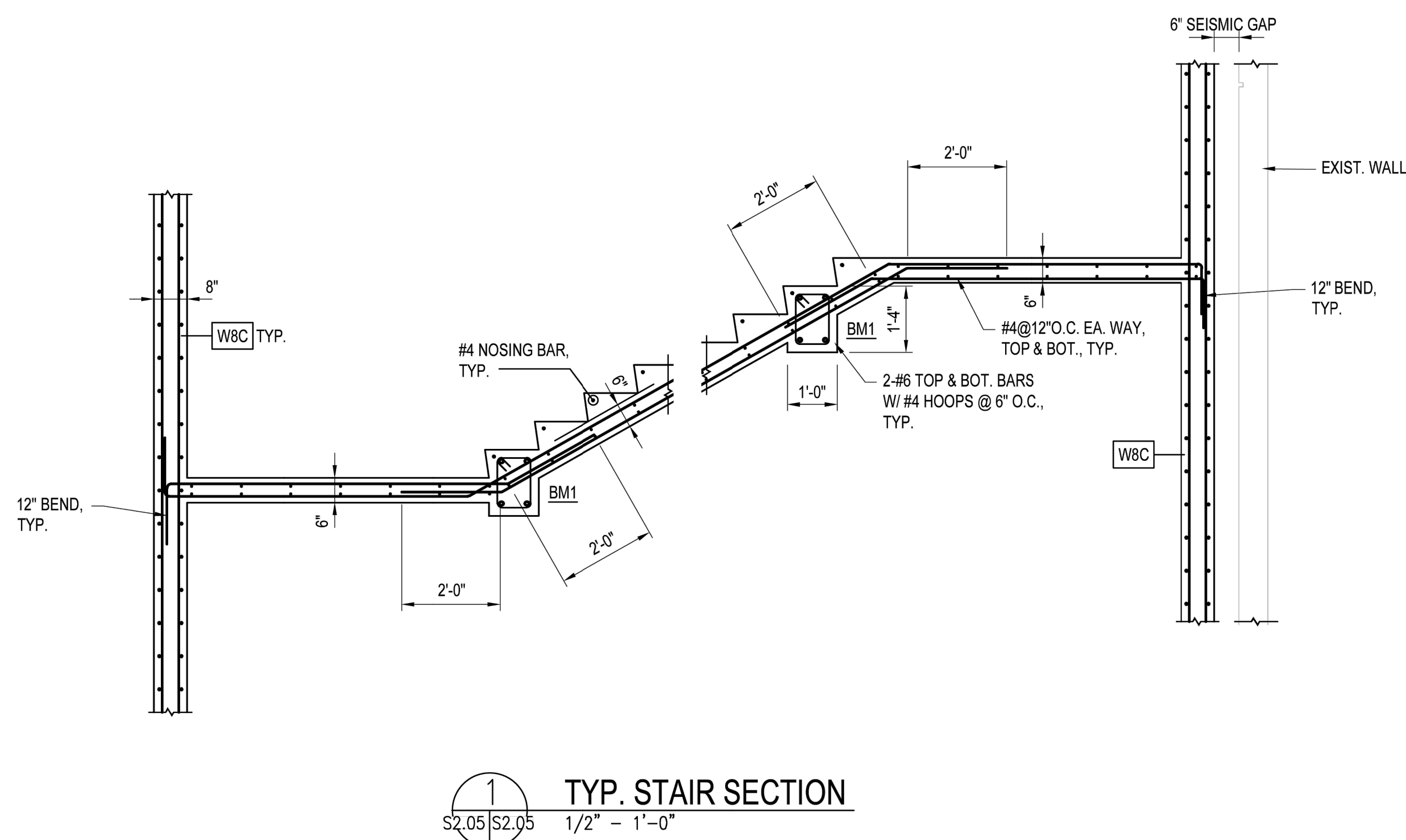
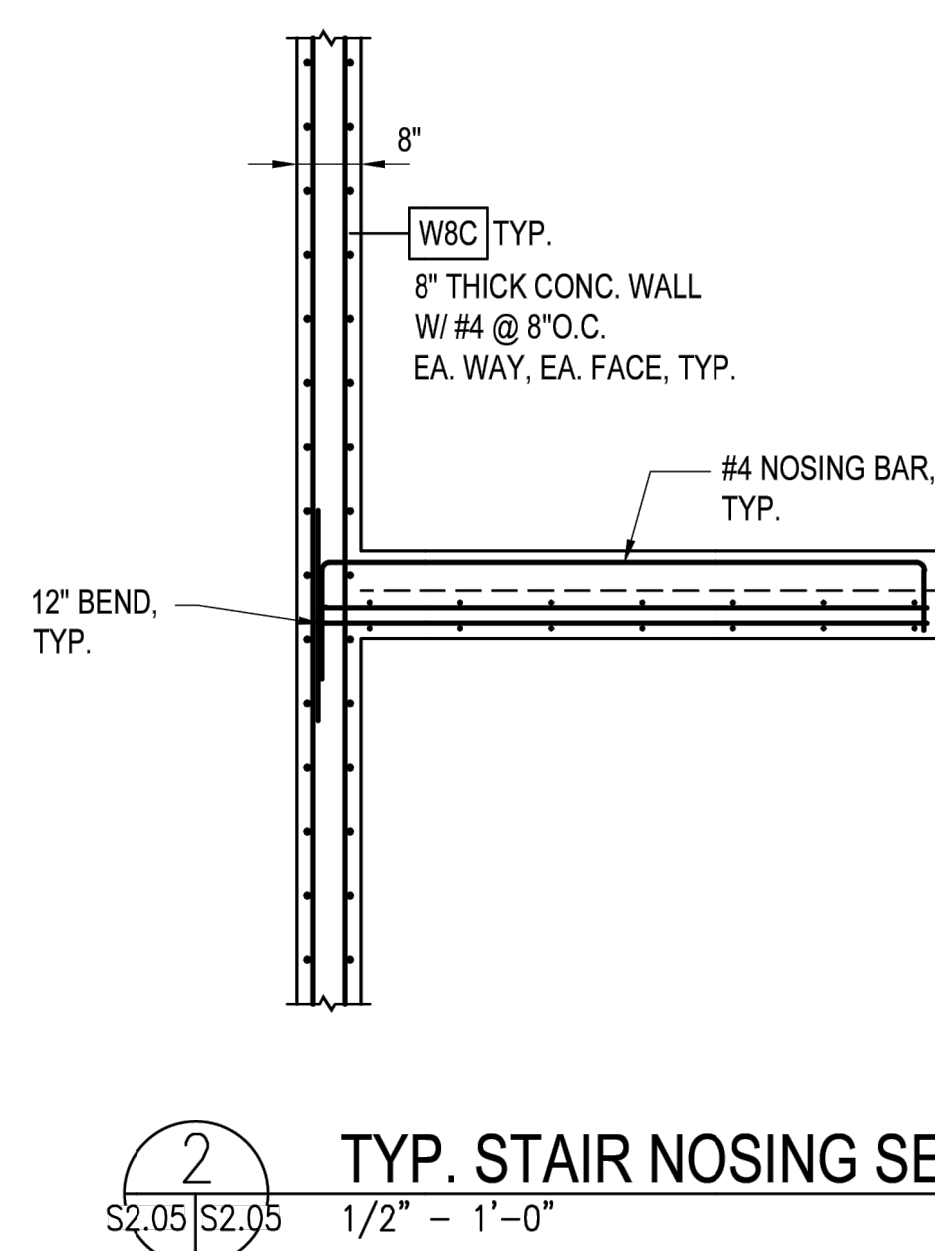
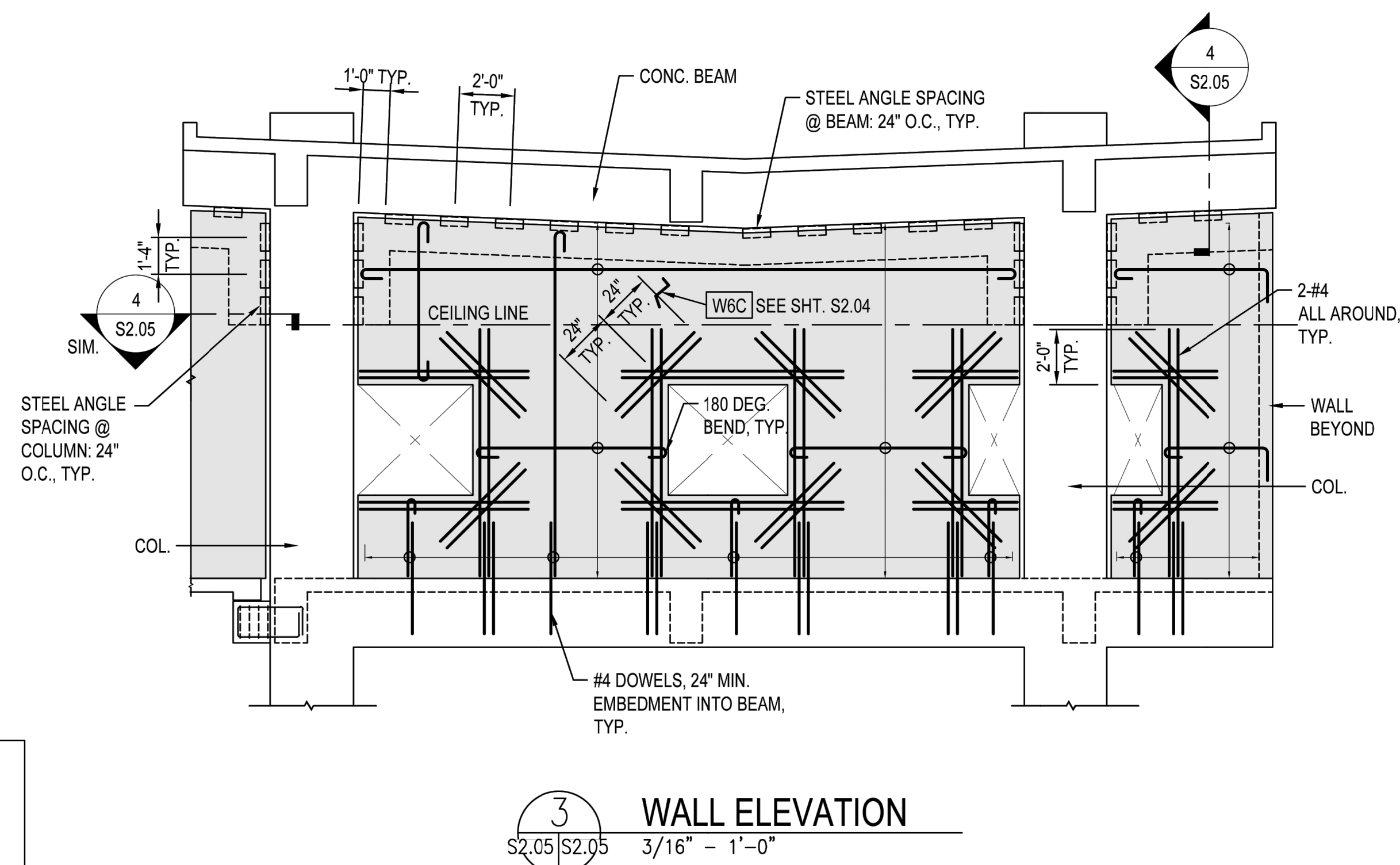
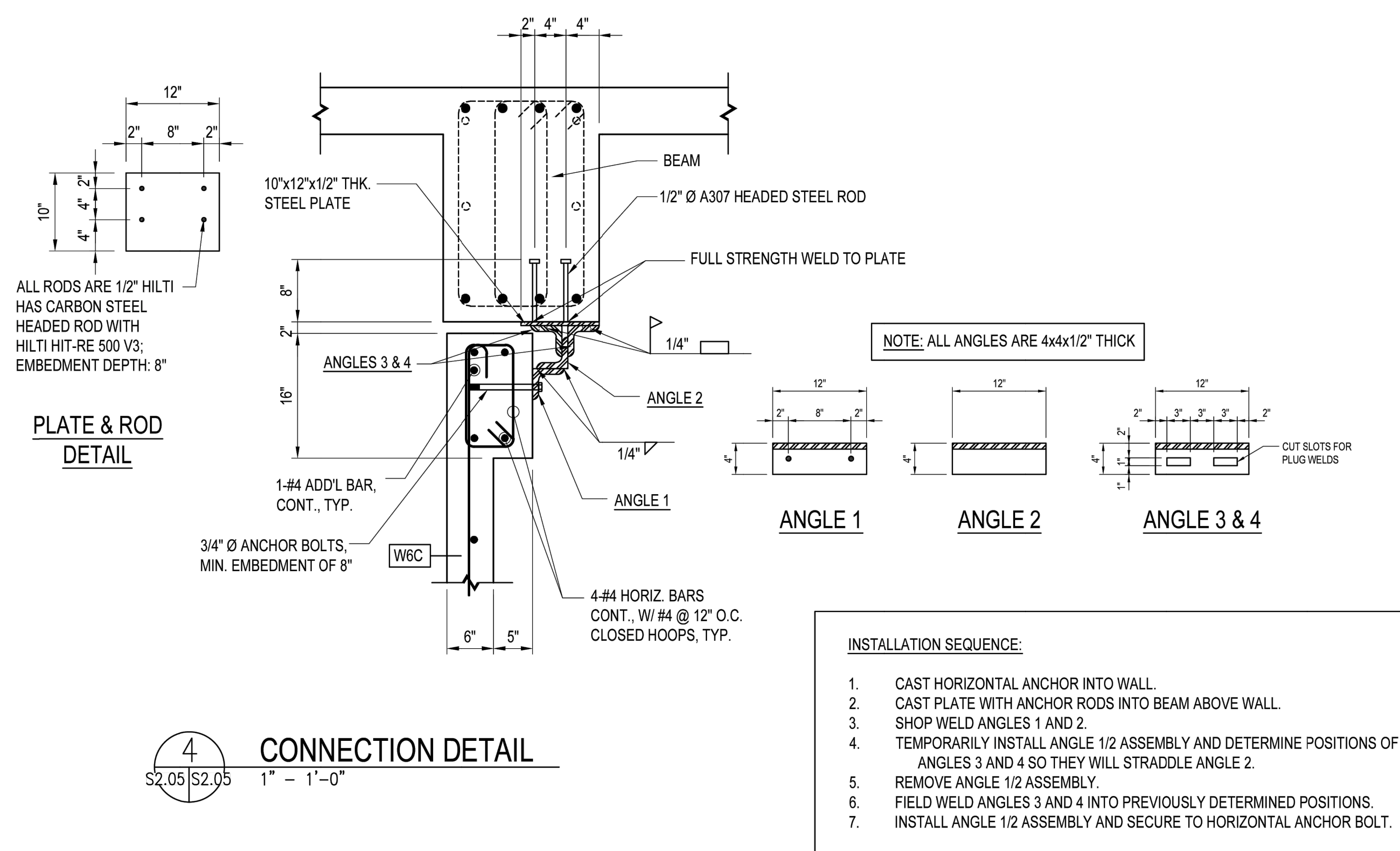


2
S2.04 | S2.04
SECTION ALONG GL - 6 & 7
3/16" = 1'-0"

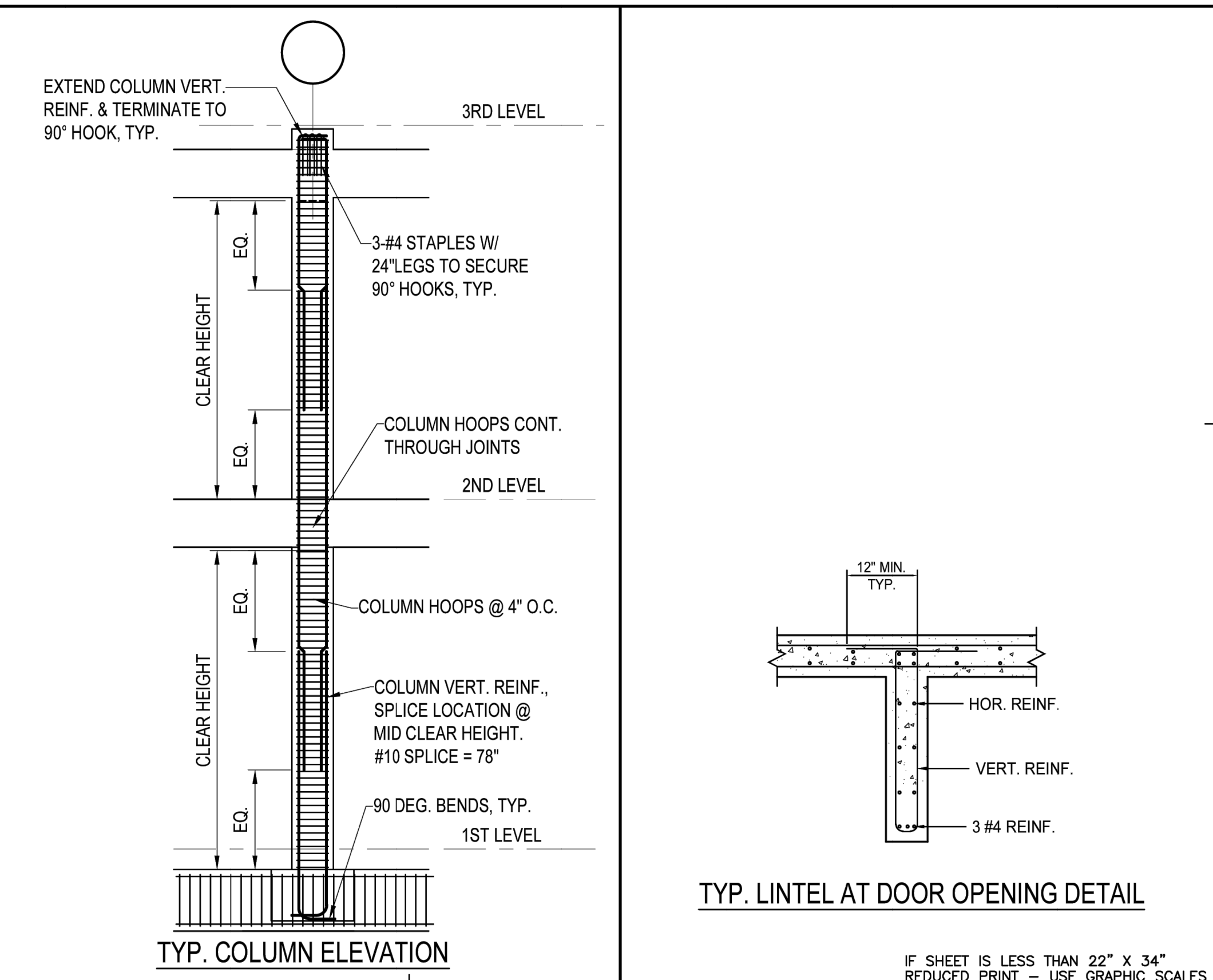


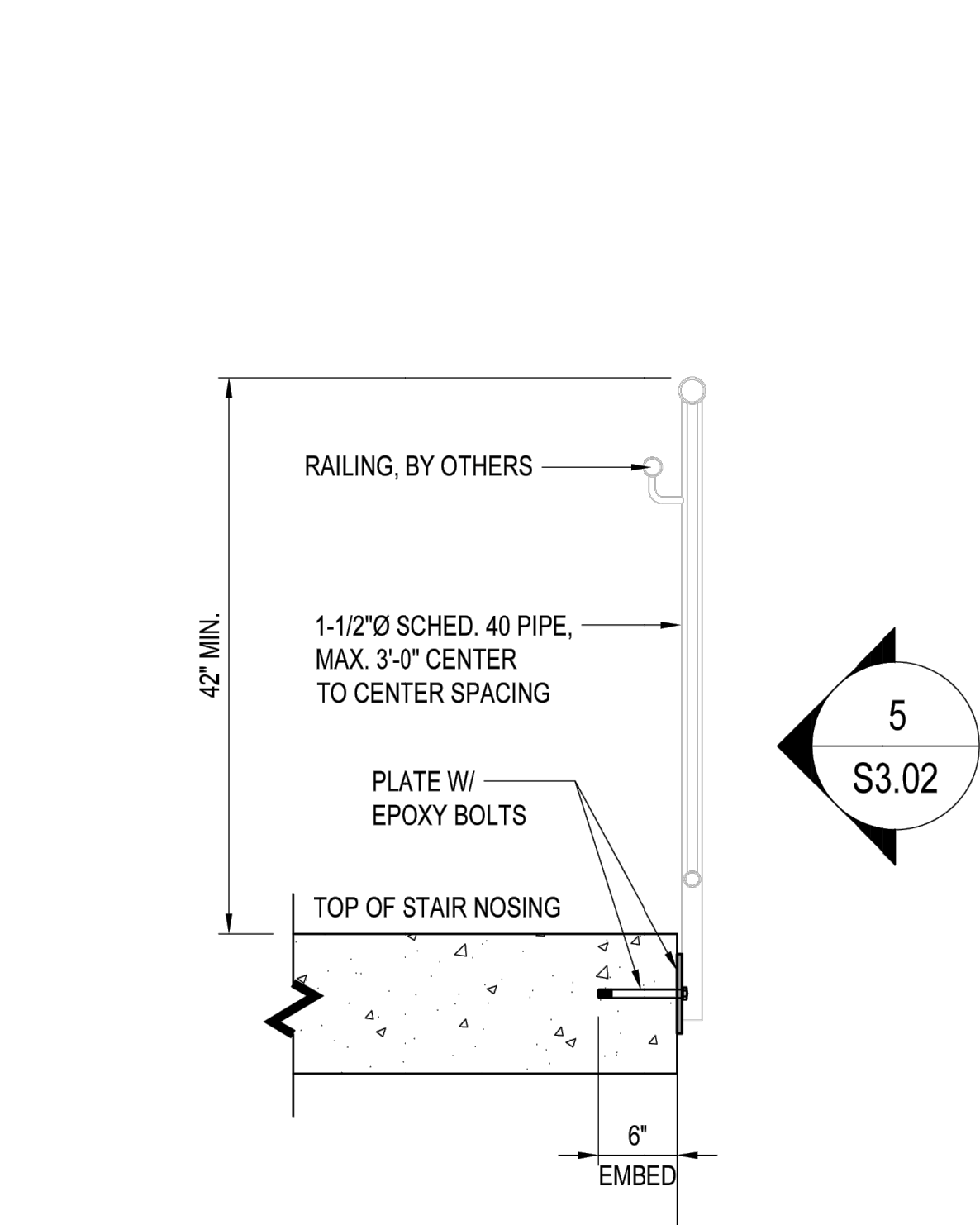
1
S2.04 | S2.04
SECTION ALONG GL - F & G
3/16" = 1'-0"

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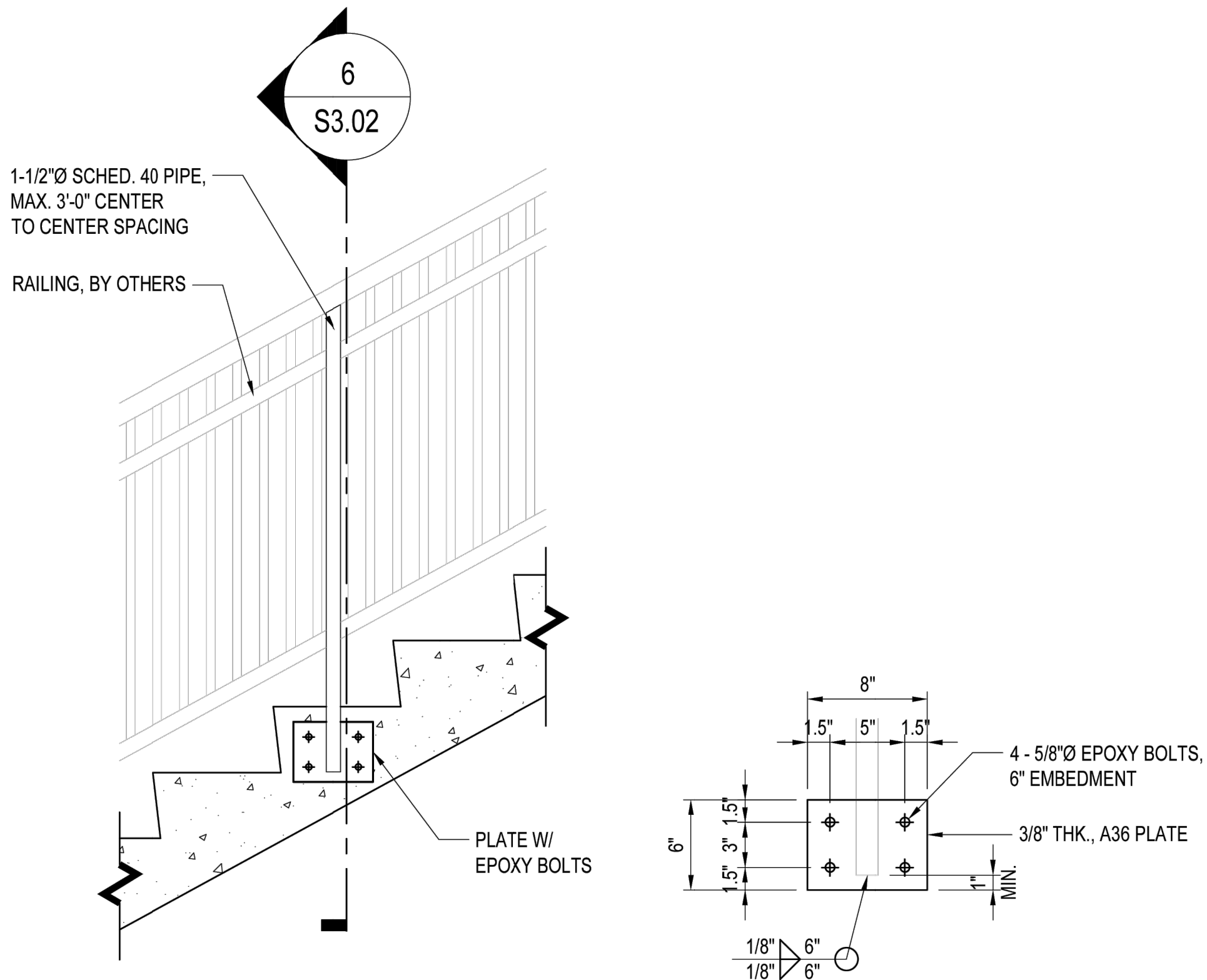


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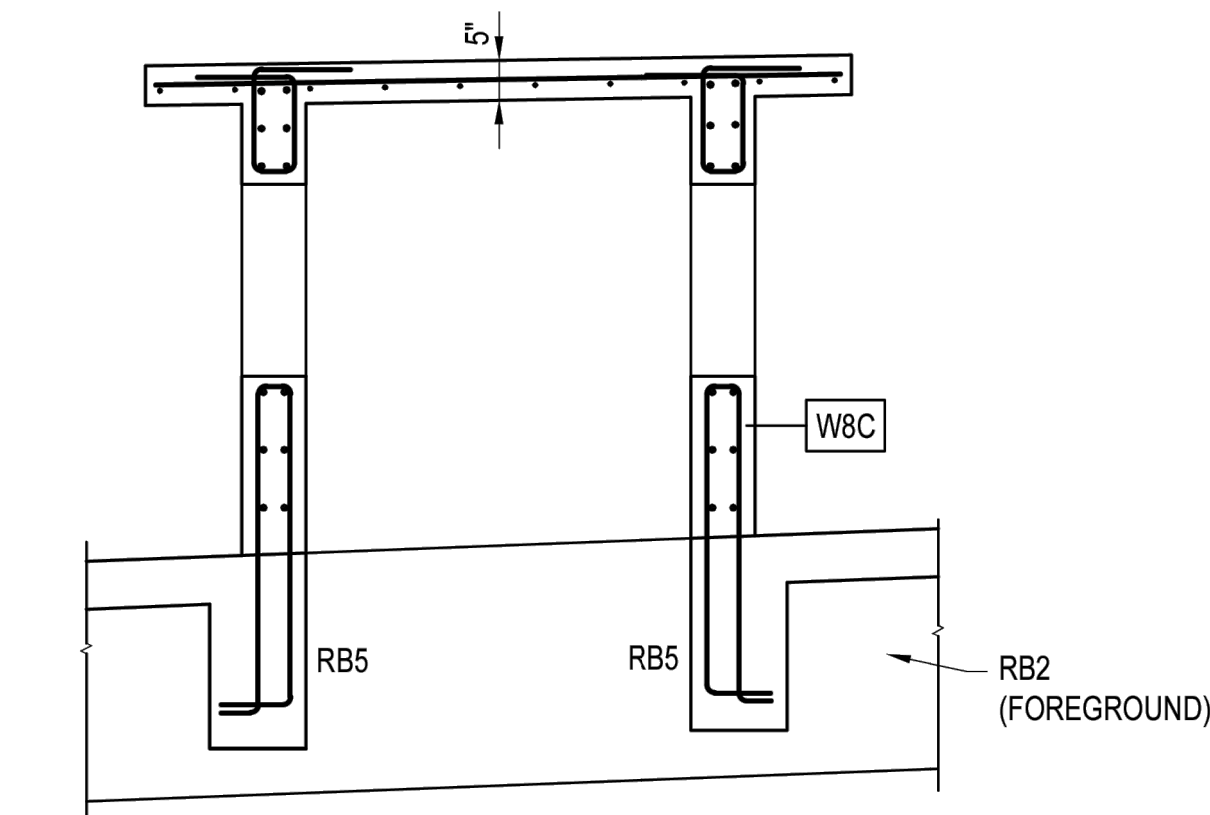




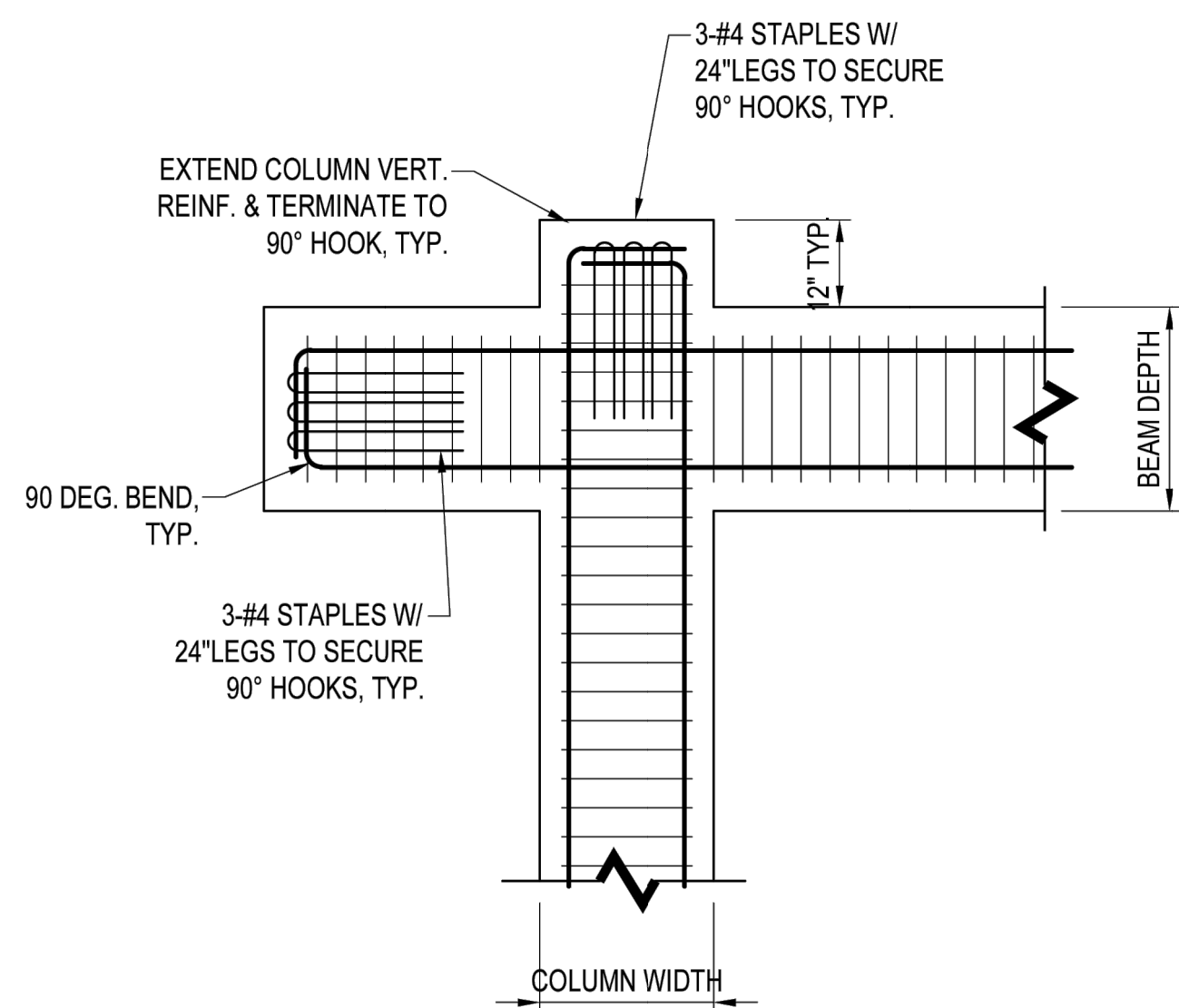
6 RAILING CONNECTION SECTION DETAIL
S3.02 | S3.02 1" = 1'-0"



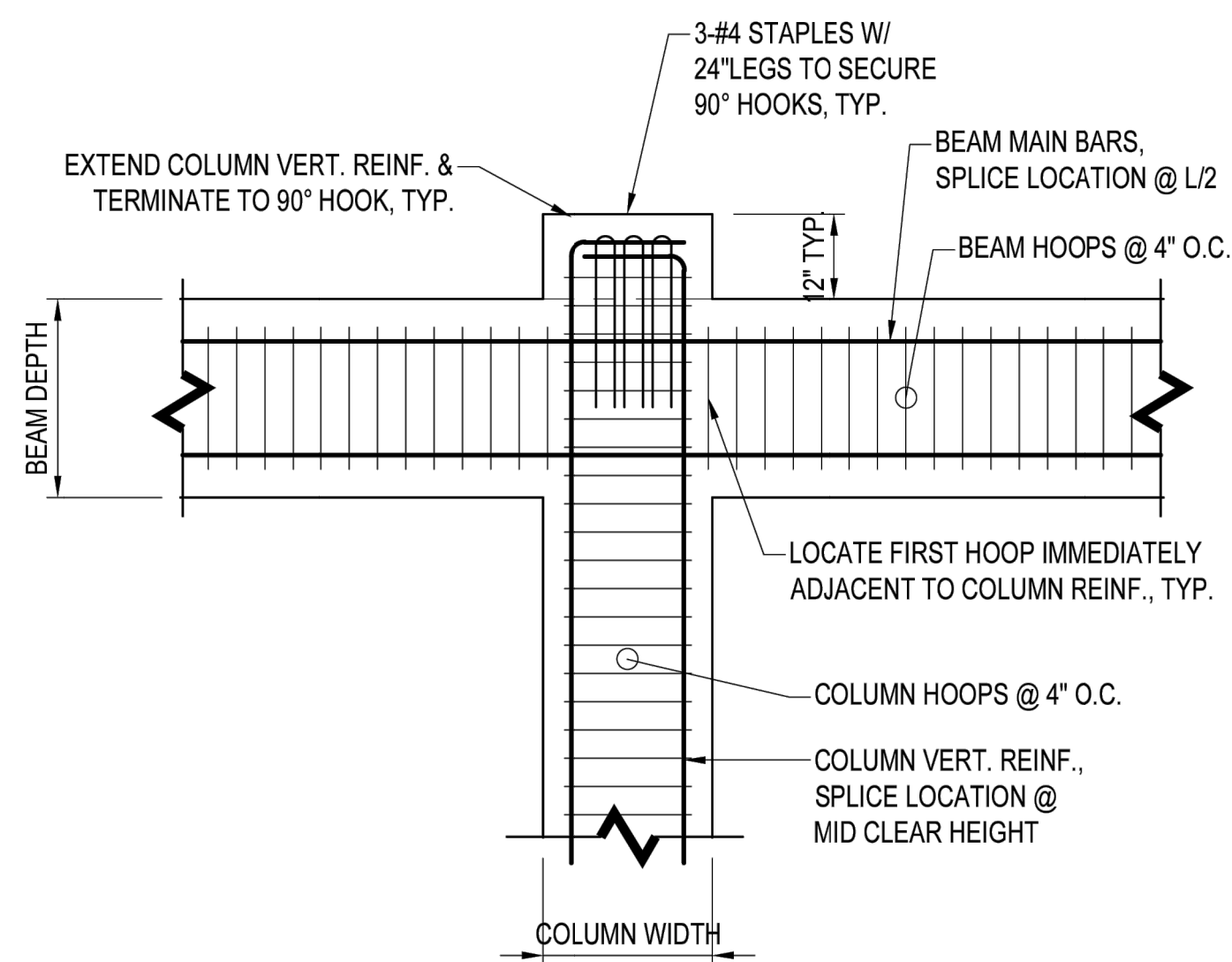
5 RAILING CONNECTION ELEVATION DETAIL
S3.02 | S3.02 1" = 1'-0"



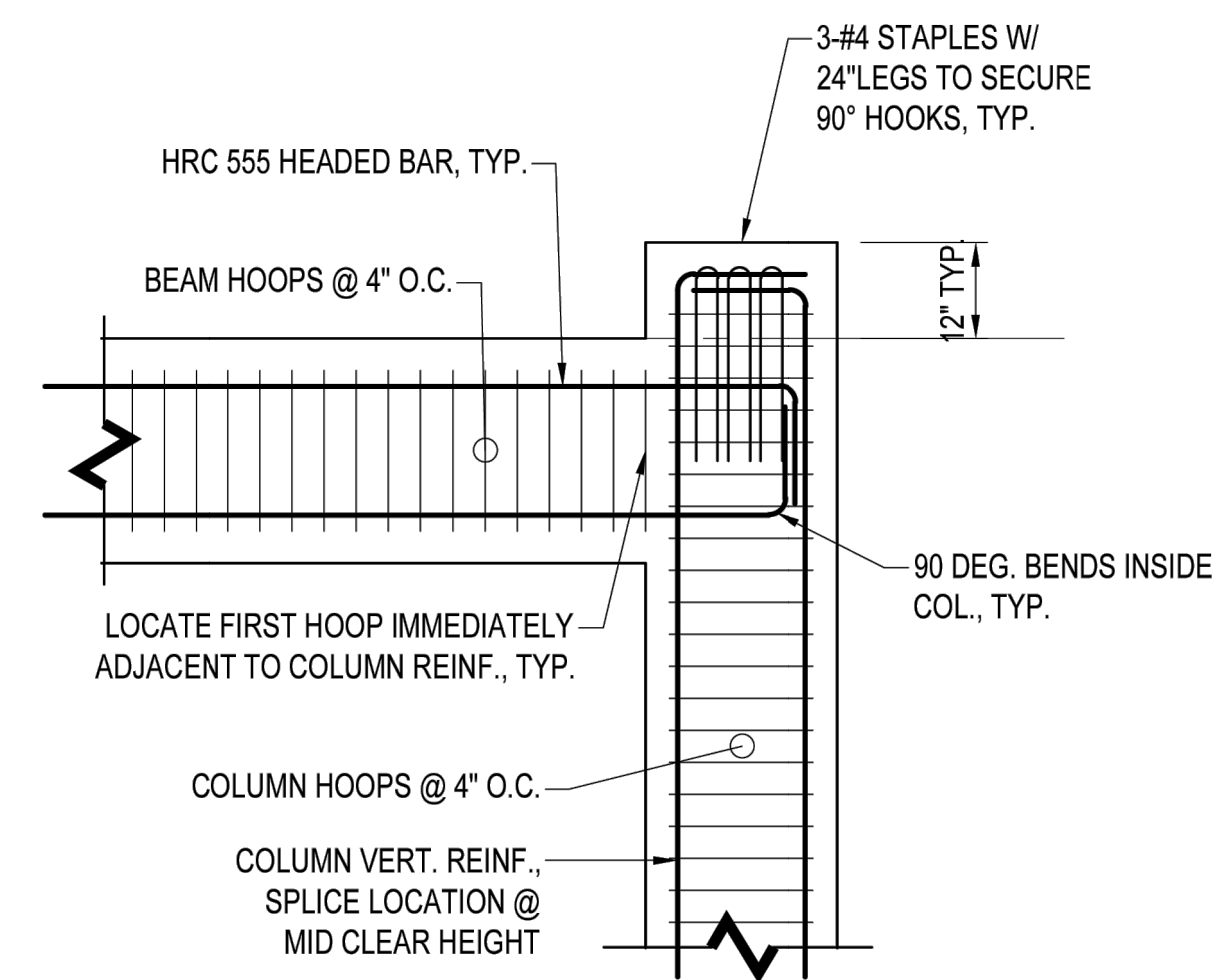
4 ROOF CLEARSTORY SECTION
S3.02 | S3.02 1/2" = 1'-0"



3 TYPICAL COLUMN STUB OUT AT BEAM OVERHANG DETAIL
S3.02 | S3.02 1/2" = 1'-0"



2 TYPICAL COLUMN STUB OUT DETAIL
S3.02 | S3.02 1/2" = 1'-0"



1 TYPICAL COLUMN STUB OUT AT EXTERIOR COLUMN
S3.02 | S3.02 1/2" = 1'-0"

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